

GEOTECHNICAL INVESTIGATION CULVERT REPLACEMENT REID SIDEROAD MILTON, ONTARIO

Submitted to: TOWN OF MILTON INFRASTRUCTURE DEPARTMENT 150 Mary Street Milton, Ontario L9T 6Z5 Attention: Mr. John Brophy, Director of Infrastructure

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Table of Contents

1.	AUI	THORIZATION
2.	INT	RODUCTION
	2.1.	SITE AND PROJECT DESCRIPTION
	2.2.	TERMS OF REFERENCE
	2.3.	GEOLOGICAL SETTING
3.	FIEL	D AND LABORATORY INVESTIGATION
	3.1.	FIELD WORK
	3.2	PHYSICAL LABORATORY WORK
	3.3	ENVIRONMENTAL SAMPLE COLLECTION AND ANALYSIS
	3.3.1.	SITE CONDITION STANDARDS
	3.3.2.	SOIL SAMPLING, INSPECTION AND PRESERVATION PROCEDURES4
4.	GEN	IERAL SUBSURFACE CONDITIONS
	4.1	GROUNDWATER CONDITIONS
5.	DIS	CUSSION AND RECOMMENDATIONS
	5.1.	CULVERT FOUNDING CONDITIONS
	5.2.	EXCAVATIONS
	5.3.	GROUNDWATER DURING CONSTRUCTION
	5.4.	EROSION CONTROL
	5.5.	RE-USE OF EXCAVATED MATERIAL AS MASS FILL AND BACKFILL
	5.6.	ENVIRONMENTAL TESTING RESULTS AND CONSIDERATION9
6.	CLO	SURE



APPENDIX A: FIGURES AND BOREHOLE LOGS FIGURE 1 – SITE LOCATION PLAN FIGURE 2 – BOREHOLE LOCATION PLAN FIGURE 3 – PARTICLE SIZE ANALYSIS REPORT FIGURE 4 – PARTICLE SIZE ANALYSIS REPORT EXPLANATION OF BOREHOLE LOGS BOREHOLE LOGS APPENDIX B: ENGINEERED FILL SPECIFICATIONS APPENDIX C: ANALYTICAL LAB RESULTS APPENDIX D: REPORT LIMITATIONS





1. AUTHORIZATION

Formal authorization to proceed with this investigation was received from the Town of Milton, Infrastructure Department, ("Client") in the form of a purchase order, number C-18-37.

2. INTRODUCTION

Wood Environment & Infrastructure Solutions, a Division of Wood Canada Limited ("Wood"), was retained by the Client to conduct a geotechnical investigation for the proposed replacement culvert located on Reid Sideroad approximately 400 m west of Stokes Trail in Milton, Ontario. The overall site location is shown on Figure 1 in Appendix A.

2.1. SITE AND PROJECT DESCRIPTION

The existing Reid Sideroad 900 mm diameter culvert consisted of a corrugated steel pipe (CSP) at the upstream end and a PVC pipe at the down-stream end. The culvert which is situated approximately 3 m below road grade is scheduled for replacement with a larger diameter concrete culvert, the size and invert grades of which were not available at the time of report preparation.

The following report sections provide a summary of the findings of the field investigation, and provide geotechnical engineering recommendations pertaining to subgrade preparation, soil foundation bearing values, excavations during construction, soil reuse and groundwater conditions.

2.2. TERMS OF REFERENCE

The findings of the investigation, together with Wood's comments and recommendations, are presented in this report. The anticipated construction conditions are also discussed but only to the extent that they may influence the design decisions. Any construction methods discussed express Wood's opinions only and are not intended to direct contractors on how to carry out the construction. Contractors should also be aware that the data and the interpretation presented in this report may not be sufficient to assess all the factors that may have an effect on construction.

This report was prepared with the assumption that the design will be in accordance with applicable standards and codes, regulations of authorities having jurisdiction, and good engineering practices.

Further, the recommendations and opinions expressed in this report are only applicable to the proposed project as described above.

An ongoing liaison with Wood must be maintained during both the design and construction phases of the project to ensure that the recommendations in this report have been interpreted and implemented correctly. Also, if any further clarification and/or elaboration are needed concerning the geotechnical aspects of this project, Wood should be contacted immediately.





2.3. GEOLOGICAL SETTING

Map 2509 - Quaternary Geology of the Hamilton Area Southern Ontario published by the Ontario Ministry of Northern Development and Mines, indicates that surficial soils in the project area are likely to consist of sand and gravel associated with kame and esker formations.

Map 2336 - Paleozoic Geology of the Hamilton Area published by the Ontario Division of Mines, indicates the bedrock in the project area consists of blue – grey dolostone of the Amabel Formation.

Map P.495 – Southern Ontario Hamilton Sheet Drift Thickness Series published by the Ontario Department of Mines indicates the dolostone can be expected to be located more than 8 m below grade.

3. FIELD AND LABORATORY INVESTIGATION

The investigation was carried out to obtain information about the soils at this site by means of sampled boreholes and laboratory testing.

3.1. FIELD WORK

A total of two (2) boreholes were drilled on January 4th, 2019. The locations of the sampled boreholes are indicated on the attached Borehole Location Plan (Figure 2).

Drilling operations were performed by Davis Drilling Inc. using a truck mounted drill rig. The rig was outfitted with 150 mm solid stem augers.

Boreholes BH1 and BH2 were drilled on Reid Sideroad, with BH1 in the west bound lane and BH2 in the east bound lane, both to a depth of 8.2 m below existing grade.

All soil samples and auger cuttings were visually examined and classified in the field. Prior to carrying out any fieldwork, the appropriate utility companies were notified to carry out underground service clearances at the borehole locations.

The drilling, sampling and testing operations were conducted under the direction of a qualified Wood geotechnical personnel who logged the various soil strata. All soil samples and auger cuttings were visually examined and classified in the field. Groundwater and drilling conditions as well as any pertinent subsurface observations were also recorded during drilling progression.

Samples were obtained through the overburden soil by driving a split spoon sampling device in accordance with the requirements of the Standard Penetration Test (SPT), ASTM D-1586. The number of blows required to drive the sampler for 0.3 m of penetration was recorded.

Upon completion, Boreholes BH1 and BH2 were backfilled to grade with bentonite in accordance with the requirements of Ontario Regulation 903, as amended.

Groundwater conditions were measured during and upon completion of drilling.





3.2 PHYSICAL LABORATORY WORK

The soil samples were returned to Wood's Burlington laboratory for further visual examination and classification. The following analyses were performed:

- Water content determination on each of the collected soil samples
- Two (2) grain size distribution analysis on representative samples

All soil samples will be stored for three months upon completion of this report. The samples will then be discarded unless Wood is instructed otherwise.

3.3 ENVIRONMENTAL SAMPLE COLLECTION AND ANALYSIS

The environmental component of the subsurface investigation included the following activities:

- Conducting the soil sampling activities in accordance with the Ministry of the Environment (MOE) document entitled "Guide for Completing Phase Two Environmental Site Assessments under Ontario Regulation 153/04" dated June 2011, the Ministry of the Environment and Energy (MOEE) document entitled "Guidance on Sampling and Analytical Methods for Use at Contaminated Sites in Ontario", dated December 1996; and MOE document entitled "Protocol for Analytical Methods Used in the Assessment of Properties under Part XV.1 of the Environmental Protection Act" issued by the Laboratory Services Branch of the MOE and dated March 9, 2004, amended as of July 1, 2011 (Analytical Protocol). It should be noted that the MOE has recently been renamed the Ministry of Environment, Conservation and Parks (MECP);
- Submission of two (2) soil samples (1 from each borehole) for laboratory analysis of metals and inorganics so as to assist in determining appropriate soil disposal options, if required, during construction;
- Submission of one (1) soil sample for Ontario Regulation 347 (O. Reg. 347) as amended by Ontario Regulation 558/00 (O. Reg. 558/00) Toxicity Characteristic Leaching Procedure (TCLP) for volatile organic compounds (VOCs), benzo(a)pyrene (B(a)P) and metals and inorganics to determine landfill acceptability of soil/fill originating from the Site;
- Comparing laboratory analytical results to the soil and groundwater outlined in the MECP "Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act," dated April 15, 2011 and O. Reg. 347 (as amended by O. Reg. 558/00) Schedule 4 Leachate Quality Criteria provided in the MECP document entitled "Registration Guidance Manual For Generators of Liquid Industrial and Hazardous Waste," October 2000 (the "Schedule 4 Criteria").





3.3.1. SITE CONDITION STANDARDS

Soil results were compared to the MECP Table 1 SCS for Residential / Parkland / Institutional / Industrial / Commercial / Community Property Use (Table 1 SCS) and Table 3 SCS for Industrial / Commercial / Community Property Use (Table 3 SCS) as presented in "Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act", dated April 15, 2011. Additionally, one (1) soil sample was compared to Schedule 4 Leachate Quality Criteria of O. Reg. 347 (as amended by O. Reg. 558/00)

3.3.2. SOIL SAMPLING, INSPECTION AND PRESERVATION PROCEDURES

Soil samples were obtained for laboratory analysis and field screening, where applicable, using a drill rig equipped with split spoon sampling capabilities. The drillers cleaned the split spoon by removing loose dirt from the split spoon using a wire brush, washing the split spoon using a brush in a dilute mix of potable water and Alconox soap, rinsing the split spoon with distilled water and rinsing the split spoon with methanol and allowing the split spoon to air dry.

The drillers obtained the split spoon sample by auguring to the specified depth, hammering the spoon about 0.6 m into the soil and removing the spoon. The split spoon sample was inspected for visual and/or olfactory evidence of environmental impacts. Disposable nitrile gloves were used and replaced between the handling of successive samples.

Soil samples deemed to be representative of the Site conditions were collected and placed in laboratorysupplied glass jars equipped with Teflon seals and submitted for metals & inorganics. The samples were selected on the basis of visual/olfactory evidence of impacts, field screening results, or from the vicinity of the apparent water table. All samples were stored in coolers, on ice, immediately after collection and during transport to the laboratory.

Representative soil samples collected during the investigation were submitted to AGAT Laboratories (AGAT) of Mississauga, Ontario. AGAT is accredited by the Standards Council of Canada (SCC) and the Canadian Association for Laboratory Accreditation (CALA) in accordance with ISO/IEC 17025:2005 – "General Requirements for the Competence of Testing and Calibration Laboratories" for the tested parameters set out in the Soil, Ground Water and Sediment Standards

4. GENERAL SUBSURFACE CONDITIONS

For a description of the soil conditions encountered at each location during this investigation, reference should be made to the Record of Borehole Logs in Appendix A.

Pavement Structure

Approximately 215 mm and 180 mm of asphalt was encountered in Boreholes BH1 and BH2, respectively.

Sand Fill





The asphalt in both boreholes was underlain by a sand fill deposit, which contained trace to some silt and gravel. The sand fill deposit extended to a depth of 2.1 m in Borehole BH1 and 2.2 m in Borehole BH2. The SPT 'N' values ranged from 40 to 74 blows for 0.3 m of penetration within the sand fill which indicated the fill material was in a dense to very dense condition. The natural moisture content of the sand fill deposit ranged from 5% to 12%.

Silty Clay Fill

The sand fill in both boreholes was underlain by a silt clay fill deposit, which contained trace to some sand and gravel. This silty clay fill deposit extended to 3.8 m and 3.7 m below surface grade in Boreholes BH1 and BH2, respectively. The SPT 'N' values ranged from 4 to 12 blows for 0.3 m of penetration which indicated firm to stiff consistencies. The natural moisture content ranged from 11% to 16%.

Peat

The silty clay fill in both boreholes was underlain by a deposit of peat, which extended to 4.1 m and 4.0 m below surface grade in Boreholes BH1 and BH2, respectively. The natural moisture content of the peat was determined to be 128%.

Cobbles and Gravel

The peat in Borehole BH2 was underlain by a deposit of cobbles and gravel which extended to a depth of 4.1 m below existing grade.

Silt and Sand

The peat in Borehole BH1 and the cobbles and gravel in Borehole BH2 were underlain by a silt and sand deposit, which contained some clay, trace gravel and organics. The silt and sand deposit extended to a depth of 5.3 m in Borehole BH1 and 5.2 m in Borehole BH2. The SPT 'N' values of 3 and 5 blows for 0.3 m of penetration within the silt and sand indicated the material was in a loose condition. The natural moisture content of the silt and sand deposit was determined to be 28% and 30%.

The result of one grain size / hydrometer analysis of the selected sample is summarized in the following table. The results are also noted in Figure 3.

		Depth		Particle Size	e Distributio	on
BH ID	Sample	(m)	Gravel	Sand	Silt	Clay
BH1	SS6B & SS7	4.1 to 5.2	6.5%	37.8%	43.6%	12.1%

Table 1: Summary of Grain-Size Distribution

Sand

The silt and sand deposit in both boreholes was underlain by a sand deposit, which contained some silt and gravel and trace clay. The sand deposit extended to at least the borehole termination depths of 8.2 m below existing grade. The SPT 'N' values within the sand deposit ranged from 7 to 12 blows for 0.3 m of





penetration which indicates the material was in a loose to compact condition. The natural moisture content of the sand deposit ranged from 5% 17%.

The result of one grain size / hydrometer analysis of the selected sample is summarized in the following table. The results are also noted in Figure 4.

BH	Sample		F	Particle Size	e Distributio	on
ID		Depth (m)	Gravel	Sand	Silt	Clay
BH2	SS8	5.3 to 5.9	22.4%	53.3%	20.1%	4.2%

Table 2: Summary of Grain-Size Distribution

4.1 GROUNDWATER CONDITIONS

Upon completion of the boreholes, the groundwater level was measured to be 4.1 m and 2.8 m below existing grade in Boreholes BH1 and BH2, respectively. Additionally, the borehole cave in depths upon completion were measured to be at 4.6 m and 6.7 m below existing grade in Boreholes BH1 and BH2, respectively.

It should be noted that water levels are subject to seasonal fluctuations and weather conditions. Groundwater levels can be expected to be somewhat higher during the spring months, and in response to major weather events and will likely match the creek water levels.

5. DISCUSSION AND RECOMMENDATIONS

The project will consist of replacing the existing 900 mm diameter culvert located approximately 3 m below the Reid Sideroad surface with a larger diameter concrete culvert.

5.1. CULVERT FOUNDING CONDITIONS

Based on the soil profile observed at the borehole locations and assuming the invert level for the proposed culvert likely match that of the existing culvert, the founding subgrade for the culvert would be within the fill soils which are underlain by the peat deposit. The fill soils and underlying peat are not considered suitable to support the proposed replacement culvert. Unacceptable settlements could occur if the culvert is placed on these soils.

The culvert will have to be founded below the fills and peat into the loose silt and sand / cobbles and gravel present at elevations of 289.6 m and 287.9 m in BH1 and BH 2, respectively, and may be designed to a factored ultimate limit state (ULS) bearing value of 150 kPa, and serviceability limit state (SLS) bearing value of 100 kPa. The total and differential settlements are anticipated to be less than 25 mm and 15 mm respectively.





For subgrade protection, lean concrete mud mat (or approved material) is recommended to be placed on the exposed subgrade.

Should higher bearing values be required, or the recommended founding levels be impractical for the proposed culvert, consideration can be given to full removal of any fill and peat present below the specified founding level and their replacement with either lean mix concrete fill or Engineered Fill. The engineered fill material can consist of suitable on-site materials as discussed in Section 5.5 or approved imported granular. A specification for engineered fill can be found in Appendix B.

The creek will require diversion during construction to ensure work is completed in dry conditions.

For construction of culvert foundations, Ontario Provincial Standard Specification (OPSS) 902 (*Construction Specifications for Excavating and Backfilling - Structures*) should be followed. Backfill, backfill transition and cover for the culvert should conform to Ontario Provincial Standard Drawing (OPSD) 802.031 (*Rigid Pipe Bedding*, *Backfill and Cover for Type 3 soil – Earth Excavation*)

Granular bedding material should be placed in layers not exceeding 200 mm in thickness and compacted to 100% of the material's Standard Proctor Maximum Dry Density (SPMDD).

5.2. EXCAVATIONS

Conventional heavy equipment should be capable of excavating the asphalt, fills, and peat.

All excavations must comply with the Occupational Health & Safety Act and Regulations for Construction Projects. All excavations deeper than 1.2 m must be sloped as outlined in the Act.

The fill materials are considered to be type 3 soils and the peat is considered to be a type 4 soil. If the soils are saturated at the time of construction, the condition of each soil type should be re-evaluated, and the classification revised if required.

In general, temporary excavation side slopes of 1 horizontal to 1 vertical for the proposed culvert are expected to remain stable through the overburden soils. Some flattening of excavation sides may be required if saturated soils are encountered.

If vertically cut and braced excavations through soils are used to limit disturbance to traffic, the guidelines governing trench side support are outlined in the Act.

A trench liner box is considered suitable for the safety of workers and is an approved construction technique. However, it does not provide a 'tight' soil support system and can result in soil loss from beneath the edges of the pavement along the sides of the trench.

It is recommended that qualified geotechnical personnel be present during excavation to review the conditions of the subgrade material.



5.3. GROUNDWATER DURING CONSTRUCTION

Based on the encountered soil and groundwater levels and the anticipated excavation depth of 4.1 m below the road surface, groundwater flow into the excavation is anticipated to be moderate to heavy. Due to the soils encountered at the anticipated excavation depths, dewatering using sumps would likely result in unstable excavation during construction. As such, specialized dewatering procedures may be required to control groundwater flow and maintain base and side slope stability.

As excavation is anticipated to be below the groundwater level where groundwater flow into the excavation is anticipated to be moderate to heavy, a hydrogeological investigation is recommended to further assess the dewatering requirements, and to obtain the appropriate water taking permitting (EASR / PTTW) and associated discharge permitting (if required).

All surface water should be directed away from any open excavations. A cofferdam and/or diversion of the creek flow will be required for such purpose. During construction, temporary erosion and sediment controls such as sediment trap, interceptor drain, dyke and/or silt fence should be provided and installed to prevent uncontrolled water/sediment flow into the existing water course.

5.4. EROSION CONTROL

Inlet and outlet protection, in accordance with OPSS 511 (*Rip-Rap, Rock Protection and Granular Sheeting*) and OPSD 810.010 (*Rip-Rap Treatment for Sewer and Culvert Outlets*), are recommended to prevent erosion adjacent to the culvert as well as scour that could undermine the culvert foundation. Non-woven geotextile should be placed below the rip-rap, in accordance to OPSS 1860 (*Geotextiles*), to minimize the potential for erosion of fine particles from below the inlet/outlet treatment.

The embankment slope surface should be covered with topsoil and seeded/sodded as soon as possible after completion of construction. Seeding and sodding should comply with OPSS 803 (*Construction Specification for Seed and Cover*) and OPSS 804 (*Construction Specification for Sodding*).

Temporary erosion control measures required for construction should comply with OPSS 805 (*Construction Specification for Temporary Erosion and Sediment Control Measures*).

5.5. RE-USE OF EXCAVATED MATERIAL AS MASS FILL AND BACKFILL

The material excavated from the site is anticipated to consist of asphalt, sand and silty clay fills and peat. The asphalt and peat are not suitable for re-use. The sand and silty clay fills are suitable for re-use provided the soil is clean and can be properly compacted. Generally, heavy compaction equipment must be used in order to break up and thoroughly re-compact clayey soils to prevent post-construction settlement. Poorly compacted backfill will settle and will be reflected in the surface pavement. Also, it is generally difficult to adequately place and compact cohesive soils in small, confined areas such as within narrow trenches.

The soils must also be at a suitable water content for re-compaction. Typically, the optimum moisture content for sandy and clayey soils for re-compaction are 13% and 18%, respectively. The moisture contents recorded for the sand fill ranged from 5% to 12% and from 11% to 22% for the silty clay fill. Therefore, some reconditioning may be required depending on the moisture conditions at the time of construction.





In view of the above, it may be more practical to import a granular fill where fill is needed. Imported Granular 'B' or better can be used as backfill / engineered fill. Granular fills are generally easier to place and compact in confined areas, expediting the compaction process. Any unsuitable soils encountered during the excavation should be removed from the site and disposed of appropriately, depending on the condition of the soil. The degree of compaction required in a filled location will depend on the amount of settlement that is tolerable. All fills below settlement sensitive areas should be placed in maximum 30 mm loose lifts and compacted to a minimum of 100% of Standard Proctor Maximum Dry Density (ASTM D6938).

The pavement structure in the excavated area should be reinstated to match the existing pavement structure on both sides of the culvert, or as per the Town of Milton's specifications.

5.6. ENVIRONMENTAL TESTING RESULTS AND CONSIDERATION

Wood completed an Environmental Soil Quality Testing Program as part of the Geotechnical Investigation. The details of the drilling program, including borehole locations and drilling methodology are presented in the geotechnical investigation sections of this report. Two (2) soil samples were submitted for laboratory analysis of metals and inorganics based on presence of fill material and depth of construction works. One (1) TCLP soil sample was submitted for laboratory analysis of volatile organic compounds (VOCs), benzo(a)pyrene (BaP) and metals and inorganics.

Aside from fill material (i.e., sand / silty clay) in the boreholes, no other evidence (i.e., visual/olfactory) of environmental impacts were observed in any of the soil samples collected from this project area.

All soil sample results were reported below the Table 1 SCS for metals & inorganics with the following exceptions":

• Electrical conductivity (EC) (Table 1 SCS – 0.57 millisiemens per centimeter [mS/cm]) was detected in BH1 at 1.12 mS/cm at a depth between 3.8-4.4 mbgs (sample BH1-SS6) and in BH2 at 1.19 mS/cm at a depth between 3.1-3.6 mbgs (sample BH2-SS5); and

Sodium Adsorption Ratio (SAR) (Table 1 SCS – 2.4 [unitless]) was detected in BH1 at 4.48 at a depth between 3.8-4.4 mbgs (sample BH1-SS6) and in BH2 at 3.86 at a depth between 3.1-3.6 mbgs (sample BH2-SS5).A representative soil sample was submitted for TCLP analysis of VOCs, benzo(a)pyrene and metals and inorganic parameters. The sample met the applicable Schedule 4 criteria, therefore the soil is considered non-hazardous for disposal purposes (at an MECP approved facility).

The laboratory certificates of analysis are presented in Appendix C.

The laboratory data is considered acceptable to be relied upon based on the reporting limits being met for all samples and tested parameters, no tested parameter being present in a detectable concentration in any laboratory Method Blank and all laboratory surrogates, reference materials and spikes were within acceptable limits.

It should be noted that EC and SAR are commonly associated with road salt used for de-icing activities along roads and highways. Exceedances of SAR and EC will be exempt as per Section 48 (3) of Ontario







Regulation (O. Reg.) 153/04: "If, having regard to any phase one and phase two environmental site assessments for a property, a qualified person determines that an applicable site condition standard is exceeded at the property solely because a substance has been used on a highway for the purpose of keeping the highway safe for traffic under conditions of snow or ice or both, as provided for under section 2 of Regulation 339 of the Revised Regulations of Ontario, 1990 (Classes of Contaminants — Exemptions), the applicable site condition standard is deemed not to be exceeded for the purpose of Part XV.1 of the Act. O. Reg. 153/04, s. 48 (3)". Therefore, soils only with elevated concentrations of EC and SAR within the road allowance appear to originate from de-icing activities that have occurred along the road and therefore are not considered to exceed the SCS under O. Reg. 153/04 (for re-use within the road allowance).

6. CLOSURE

The attached Report Limitations in Appendix D is an integral part of this report and should be reviewed.

We trust that this report is complete within the terms of our reference. However, should questions arise concerning this report, do not hesitate to contact us.

Sincerely,

Wood Environment & Infrastructure Solutions, a Division of Wood Canada Limited

Prepared by:

Reviewed by:

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ILL

Thomas Ring, M.A.Sc., P. Eng. Senior Geotechnical Engineer



APPENDIX A: FIGURES AND BOREHOLE LOGS



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UNIFIED SOIL CLASSIFICATION SYSTEM

Determination of Particle Size Analysis of Soils LS 702 or ASTM D7928



Wood Environment & Infrastructure Solutions, 3450 Harvester Rd., Suite 100, Burlington, ON, L7N 3W5

C	5	
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UNIFIED SOIL CLASSIFICATION SYSTEM

Determination of Particle Size Analysis of Soils LS 702



EXPLANATION OF BOREHOLE LOG

This form describes some of the information provided on the borehole logs, which is based primarily on examination of the recovered samples, and the results of the field and laboratory tests. Additional description of the soil/rock encountered is given in the accompanying geotechnical report.

GENERAL INFORMATION

Project details, borehole number, location coordinates and type of drilling equipment used are given at the top of the borehole log.

SOIL LITHOLOGY

Elevation and Depth

This column gives the elevation and depth of inferred geologic layers. The elevation is referred to the datum shown in the Description column.

Lithology Plot

This column presents a graphic depiction of the soil and rock stratigraphy encountered within the borehole.

Description

This column gives a description of the soil stratums, based on visual and tactile examination of the samples augmented with field and laboratory test results. Each stratum is described according to the *Modified Unified Soil Classification System*.

The compactness condition of cohesionless soils (SPT) and the consistency of cohesive soils (undrained shear strength) are defined as follows (*Ref. Canadian Foundation Engineering Manual*):

Compact	tness of	Consistency of	Undrained	Shear Strength
Cohesionless	SPT N-Value	Cohesive Soils	<u>kPa</u>	psf
<u>Soils</u>		Very soft	0 to 12	0 to 250
Very loose	0 to 4	Soft	12 to 25	250 to 500
Loose	4 to 10	Firm	25 to 50	500 to 1000
Compact	10 to 30	Stiff	50 to 100	1000 to 2000
Dense	30 to 50	Very stiff	100 to 200	2000 to 4000
Very Dense	> 50	Hard	Over 200	Over 4000

Soil Sampling

Sample types are abbreviated as follows:

SS	Split Spoon	TW	Thin Wall Open (Pushed)	RC	Rock Core
AS	Auger Sample	TP	Thin Wall Piston (Pushed)	WS	Washed Sample

Additional information provided in this section includes sample numbering, sample recovery and numerical testing results.

Field and Laboratory Testing

Results of field testing (e.g., SPT, pocket penetrometer, and vane testing) and laboratory testing (e.g., natural moisture content, and limits) executed on the recovered samples are plotted in this section.

Instrumentation Installation

Instrumentation installations (monitoring wells, piezometers, inclinometers, etc.) are plotted in this section. Water levels, if measured during fieldwork, are also plotted. These water levels may or may not be representative of the static groundwater level depending on the nature of soil stratum where the piezometer tips are located, the time elapsed from installation to reading and other applicable factors.

Comments

This column is used to describe non-standard situations or notes of interest.

Wood

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SMALLER THAN	ELOW "A" LINE IBLE ORGANIC ONTENT	W _L < 50%	ML	INORGANI	C SILTS AND VEF	RY FINE SANDS SLIGHT PLAST	ROCK FLOUI CITY	R, SILTY SA	NDS OF					
WEIGHT (SILTS B NEGLIG C	W _L > 50%	МН	INORGANIC	SILTS, MICACEO	DUS OR DIATOI SOILS	ACEOUS, FIN	NE SANDY (OR SILTY	CLASSIE	ICATION IS	BASED LIPO	N PLASTICI	TY CHART
HALF BY V	a" LINE GANIC	W _L < 30%	CL	INORGANIC	CLAYS OF LOW I	PLASTICITY, GF LEAN CLAY	AVELLY, SAN S	DY OR SILT	TY CLAYS,	CLASSIFICATION IS BASED UPON PLASTICITY CHA (SEE BELOW)				IT OFFICE
RE THAN I	ABOVE ", BIBLE OR	30% < W _L < 50%	CI	IN	ORGANIC CLAYS	OF MEDIUM P	ASTICITY, SI	TY CLAYS						
ILS (MOF	CLAYS	W _L > 50%	СН		INORGANIC CLA	YS OF HIGH PL	ASTICITY, FA	T CLAYS						
INED SO	SLITS & LOW "A" E	$W_{L} < 50\%$	OL	ORGA	NIC SILTS AND C	RGANIC SILTY	CLAYS OF LC	W PLASTIC	YTIY	WHENEVER	THE NATUR	E OF THE F	INES CONT	ENT HAS NOT
FINE-GRA	ORGANIC CLAYS BE LIN	W _L > 50%	ОН		ORGANIC	CLAYS OF HIC	H PLASTICIT	Y		BEEN DETE E.G SF	RMINED, IT IS A MIXTUF	IS DESIGNA RE OF SAND	TED BY THE	E LETTER "F", OR CLAY
	HIGH ORGANIC SOILS		Pt		PEAT AND	OTHER HIGHLY	ORGANIC SC	ILS		STRONG	COLOUR C	R ODOUR, A	AND OFTEN	FIBROUS
		SOIL COMPO	NENTS			60 J	Plast	icity Cha	rt for Soi	Passing 4	25 Micror	n Sieve		
FRACTION	U.S STANDARD	SIEVE SIZE	DEFINING RANGE	S OF PERCENTA NOR COMPONE	AGE BY WEIGHT NTS					W, = 50				
	COARSE	PASSING	RETAINED	PERCENT	DESCRIPTOR	50								
GRAVE		76 mm	19 mm	20-35	Y/EY	40		W	= 30		СН			
	FINE	19 mm	4.75 mm	10-20 1-10	SOME TRACE	dex, I _P						'A' Line I _P = 0.7	3 (W₁ - 2(0)
0	COARSE	4.75 mm	2.00 mm			oc In								·
SANE	MEDIUM	2.00 mm	425 µm	-		Last Plast	CL		CI	X			МН	
	FINE	425 µm	75 µm	+						OL		он		
FINES (SILI PL	OR CLAY BASED ON LASTICITY)	75 µm				10	<u> </u>		V_{\dots}					
		OVERSIZED M	ATERIAL	NOT P		0	CL-M		ML					
ROUN	DED OR SUBROUNDED: BOULDERS	COBBLES 76 mm T > 200 mm	O 200 mm	ROCK FRAGE ROCKS > 0.76 VO	MENTS > 76 mm CUBIC METRE IN LUME	0	10	20	30 40 Li	50 quid Limit,	60 WL	70 80	. 90	100
Wood Environm 3450 Har Burlingto Tel: (905 Fax: (905 www.woo	nent & Infrastru vester Road, S n, Ontario, L7N) 335-2353 5) 335-1414 odplc.com	cture Soluti ouite 100 3W5	ons	00	od.	Note 1: Sc and behav Note 2: Th range by v Engineerir	ils are cla iour. e modifyir /eight of m ig Manual	ssified a ng adject ninor con	nd descri ives usec nponents	bed accord d to define are consis	ding to th the actua stent with	eir engino al or estin the Cana	eering pr nated pe adian Fo	operties rcentage undation

RE		OF BOREH	OLE N	o.	BH1	L										
Proj	ect Number: 1	FPB188147S.2000.	1						Drilling	g Location:	BH1			_		
Proj	ect Client: 1	Town of Milton							Drilling	g Method:	150 mm So	lid Stem Augers		- W	00	d.
Proj	ect Name:	Reid Sideroad Culv	vert Replace	ment (Geoteo	hnical			Drilling	g Machine:	Truck Mount	ted Drill		-		
Proj	ect Location: F	Reid Sideroad							Date	Started:	<u>Jan 4, 19</u>	Date Completed: Ja	n 4, 19	-		
Log	ged by: 1	ГН	Comp	iled by	:	TH			Revie	wed by:	WK	Revision No.: 0,	4/24/19			
	LITHO	LOGY PROFILE		SC	DIL SA	MPL	NG			FIELD	TESTING	LAB TESTING	-			
iology Plot	D	DESCRIPTION		nple Type	nple Number	covery (%)	T 'N' / RQD (%)	PTH (m)	EVATION (m)	Penetra ○ SPT □ MTO Vane* △ Intact ▲ Remould * Underined Sh	AtionTesting PPT • DCPT Nilcon Vane* • Intact • Remould	Atterberg Limits W _p W _L Plastic Liquid * Passing 75 um (%) O Moisture Content (%)	STRUMENTATION STALLATION	COMN { GRAII DISTRII (؟	NENTS & N SIZE BUTION %)	
Lit Lit	Local Ground Surface	Elevation: 291.7 m		Saı	Sai	Re	SP	DE		20 40	60 80	20 40 60 80	NNNN NNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNN	GR SA	SI	CL
		215 mm Asphalt	291.5 0.2					Ē	-							
	Trace	Sand FILL to some silt and grave Very dense Moist	el	SS	1	75	49		291 -		0	° ₅				
				SS	2	79	58	- - 1 - -			0	° ₆				
				SS	3	79	74		290 -			o. 8				
		Brown	289.6 2.1													
	Trace t	Silty Clay FILL to some sand and grav Firm to stiff WTPL	vel	SS	4	17	11		- - 289 —	0		° 13				
				SS	5	13	4	- 3 - - -		0		• • • • • • • • • • • • • • • • • • •				
			287.9					Ē	288 -							
		Peat Brown/Grey Silt and Sand	3.8 <u>287.6</u> 4.1	SS	6	100	5	- 4 - 5	Z - = -	0			• 128			
	Some cl	lay, trace gravel, organ Loose Wet	nics .	SS	7	21	3		287 -			28		6 38	44	12
<u>ki</u>		Brown/Grey	286.4 5.3					-								
	Some	Sand e silt, gravel, trace clay Loose to compact Saturated	y .	SS	8	58	10		286 -	0		° ₁₂				
				SS	9	25	7	-	-	0		·0. 5				
				SS	10	8	10	- - - 7 - -	285 -	0		°,				
									284 -							
	Вс	prehole Terminated	283.5 8.2	SS	11	46	12	- 8	-	0		016				
Woo	d		∑ Groundw	ater de	u oth duri	l na drillin	l Ig on 1/	4/2019	at a den	th of: 4.1 m		Cave in depth after re	noval of auge	rs: 4.6 m		
Envir 3450	ronment & Infrast Harvester Road	tructure Solutions	- 0.00110W		uuii			010		••• <u>1 111</u> .				<u></u> .		
Burlin Cana Tel. N www.	ngton, Ontario, L7 ada No.: 1 (905) 335-2 woodplc.com	N 3W5 353	Borehole details a qualified Geotec commissioned an	as prese chnical E Id the ac	nted, do ingineer. company	not cons Also, bo ying'Expl	titute a ti rehole in anation o	horough Iformatio of Boreho	understa on should ole Log'.	nding of all pote be read in conju	ential conditions pre unction with the geo	esent and require interpretative as stechnical report for which it was	sistance from		Scale: Page: 1	1:47 of 1

R	ECORD	OF BORE	HOLE N	0.	BH2	2										
Pro	ject Number:	TPB188147S.200	0.1						Drilling	J Location:	BH2					
Pro	ject Client:	Town of Milton							Drilling	g Method:	150 mm So	lid Stem Augers		W	00	d.
Pro	ject Name:	Reid Sideroad Cu Investigation	lvert Replace	ment (Geoteo	hnical			Drilling	g Machine:	Truck Mount	ted Drill				•••
Pro	ject Location:	Reid Sideroad							Date	Started:	<u>Jan 4, 19</u>	Date Completed: Ja	in 4, 19			
Log	ged by:	тн	Comp	iled by	r:	TH			Revie	wed by:	WK	Revision No.: 0,	4/24/19			
	LITH	OLOGY PROFIL	E	SC	DIL SA	MPLI	NG			FIELD	TESTING	LAB TESTING	-			
					5		(%)		Ê	Penetra O SPT □	tionTesting	Atterberg Limits	ATIO	COMN	ENTS	
Plot		DESCRIPTION		ype	Iumbe	(%)	RQD	Ê	NO	MTO Vane*	Nilcon Vane*	Plastic Liquid	ATIO	GRAIN		
ology				nple T	nple N	pvery	/.N.	PTH (EVAT	▲ Remould	Remould	 Passing 75 um (%) Moisture Content (%) 	TALL	0131 Kil	6)	
Lith	Local Ground Surf	ace Elevation: 291.9 m		Sar	Sar	Rec	SPI	DE	ELE	* Undrained Sh 20 40	60 80	20 40 60 80	SN GI	R SA	SI	CL
	~	180 mm Asphalt	291.7 0.2					È								
	Tra	Sand FILL ce to some silt and gra	ivel	SS	1	75	48	-			0	° ₅				
		Dense Moist							-							
				~~	2	75	50	- 1	291 -		<u>^</u>					
				00	2	15	50	_			0	1 2				
								-								
								-								
				SS	3	75	40	2	290 -			⁰ 10				
		Brown	289.7					ŀ	-							
	Trac	Silty Clay FILL e to some sand and gr	ravel					_	-							
		Firm to stiff WTPL		SS	4	63	12		Z	0		°13				
								- 3	289 -							
								Ļ								
				SS	5	50	9	-		0		°16				
		Peat	288.2 3.7					Ē								
1			287.9					÷,	288 -							
Р		Brown	28 7 :8 4.1	SS	6	92	31	- 4		0		o 18				
IH	Some	Silt and Sand clay, trace gravel, org	anics					-	-							
HD		Wet						-	-			· · · · · · · · · · · · · · · · · · ·				
				SS	7	63	5	-	287 -	0		30				
Ш		Davia	286.7					- 5								
 ₩	50	Brown Sand me silt gravel trace of	5.2 lav					-								
=		Loose to compact Saturated		SS	8	58	8	_		0		°_17	23	53	20	4
= -								-	286 —							
= = =								- 6 -								
				SS	9	63	7	-		0		0,12				
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= =				SS	10	63	12	_	284 -			0				
= =			283 7					8				11				
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Woo	pd			ater de	l oth duri	l na drillin	l Ia on 1//	1/2010	at a den	th of: 27 m		Cave in depth after re-	moval of augers	67 m		
Env 345	ironment & Infra 0 Harvester Roa	astructure Solutions		ator ue	par uurii		.g 011 <u>1/4</u>	013	ata uep	vi. <u>£.<i>í</i> III</u> .				<u></u> .		
Burl Can	ington, Ontario, ada	L7N 3W5	Borehole details	as prese	nted, do	not const	titute a th	orough	understa	nding of all pote	ntial conditions pre	esent and require interpretative as	sistance from		Scale:	1 · 47
Геl. www	No.: 1 (905) 335 v.woodplc.com	-2353	commissioned ar	nd the ac	company	/ing'Expl	anation o	of Boreh	ole Log'.	Se reau in conju	incuon with the get	Accontinuen report for Which it Was			Page: 1	of 1

APPENDIX B ENGINEERED FILL SPECIFICATIONS



The engineered fill construction should be monitored and certified by the geotechnical engineer. Construction should be carried out according to the following procedure:

- (i) The boundaries of the area to receive the base of the engineered fill should be determined and staked by proper surveying techniques to ensure that the engineered fill covers the design area.
- (ii) The base of the engineered fill must extend beyond the edge of footing (inside and outside edge), a distance equal to the depth of the engineered fill plus 1.0 metre.
- (iii) This specification covers only in-ground engineered fill and not building up the grade. If grade build up on any side of the foundation produces a slope (embankment), then a slope stability analysis and foundation assessment should be carried out in addition to these specifications.
- (iv) Also, if the finished grade of the area will be raised in excess of 1.0m, then long term consolidation settlement analysis should be performed.
- (v) The area to receive the engineered fill should be stripped of all softened clays, vegetation cover, frozen materials, weak and deleterious materials. After stripping, the entire area should be inspected and approved by the geotechnical engineer. Spongy, wet or soft/loose spots should be sub-excavated, to expose stable subgrade, and replaced with compactable approved soil, compatible with subgrade conditions, as directed by the geotechnical engineer.
- (vi) If granular material is used as engineered fill over clayey subgrade, then the subgrade should be graded and sloped to discourage ponded water. Subdrains may be also required depending on the thickness of the fill and its proposed use.
- (vii) The material used for the engineered fill must be uniform & homogeneous. It should be placed in thin lifts not exceeding approximately 300 mm when loose. 150 mm lifts should be used in confined areas where heavy compacting equipment can not be used. Oversize particles (cobbles and boulders) larger than half the lift thickness should be discarded. Excavated native soils free of significant amount of organics or deleterious materials may be suitable as engineered fill materials provided that their water contents are acceptable and the soils are not frozen. Alternatively, OPSS Granulars A and B, OPSS Select Sub-grade, or approved equivalent may be used.
- (viii) It may be possible to use fine grained cohesive soils (clay) with moisture contents that are close to optimum as engineered fill in mass fill placement subject to some restrictions in the thickness and time of construction. It is generally very difficult (and in some circumstances) impossible to compact clayey soil in wet or cold weather especially to a density that is above 95% of its Standard Proctor Maximum Dry Density (SPMDD). Also, heavy sheepsfoot non-vibratory equipment must be used in order to break-up and thoroughly re-mould/compact clay soils to achieve adequate compaction. Also, it is generally difficult to adequately place and compact cohesive soils in confined areas such as trenches or foundation excavations. Under such conditions, imported granular materials may be the only option.
- (ix) Each lift should be uniformly compacted with heavy compactors, suitable for the type of fill used to at least 100% of its SPMDD for engineered fill that will support foundations. In addition, if clay is used beneath a footing, it is recommended that there be a wait of at least three months between placement of the fill and pouring the footings. The wait is based on an engineered fill up to 1.0 m in thickness. For a thicker fill supporting settlement sensitive structure, the wait can be longer and should be assessed on a site by site basis.
- (x) The degree of compaction required in a filled location will depend on the amount of settlement that is tolerable. All fills below pavement, curb & gutter and sidewalk slab areas should be placed in maximum 300 mm loose lifts and compacted to 95% of the SPMDD. For the upper 0.6 m below the subgrade, the degree of compaction should be increased to 98%. For concrete slabon-grade, all suitable fill should be placed in maximum 300 mm loose lifts and compacted to 98 % of the SPMDD. If a settlement of 1% of the fill thickness becomes a concern for a specific slabon-grade application, then the compaction should be increased to 100% of SPMDD. In addition, if clay is used, an appropriate wait time should be accounted for to minimize any adverse effect of post construction settlement of clayey soils.
- (xi) Full-time geotechnical inspection and quality control (by means of frequent field density and laboratory testing) are necessary for the construction of a certifiable engineered fill. The compaction procedure and test frequency should be controlled by the geotechnical engineer.



- (xii) The engineered fill should not be frozen and should be placed at water contents within 2 % of the optimum value for compaction. The engineered fill should not be placed during winter months when freezing ambient temperatures occur persistently or intermittently.
- (xiii) Footings and foundation walls constructed partially or entirely on engineered fill will require reinforcing steel. As a minimum, two 15M bars should be placed in the footings and two 15M steel bars at the top of the foundation wall.
- (xiv) Spread and/or strip footings founded on engineered fill may be designed using 150 kPa for factored ULS and 100 kPa for SLS bearing values if clayey soil is used to build the fill. If OPSS Granular A is used, then 300 kPa for factored ULS and 200 kPa for SLS bearing values can be used. Bearing values for other materials must be provided by the geotechnical engineer.

HT/PM/MP/PB

APPENDIX C ANALYTICAL LAB RESULTS



Page 1 of 9

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS 3450 HARVESTER ROAD, SUITE 100 BURLINGTON, ON L7N 3W5 (905) 335-2353

ATTENTION TO: Willie Kokotec

PROJECT: TPB1881457.2000.1

AGAT WORK ORDER: 19T428150

SOIL ANALYSIS REVIEWED BY: Amanjot Bhela, Inorganic Supervisor

TRACE ORGANICS REVIEWED BY: Oksana Gushyla, Trace Organics Lab Supervisor

DATE REPORTED: Jan 21, 2019

PAGES (INCLUDING COVER): 9

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

<u>NOTES</u>

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.

AGAT Laboratories (V1)

Member of: Association of Professional Engineers and Geoscientists of Alberta (APEGA) Western Enviro-Agricultural Laboratory Association (WEALA) Environmental Services Association of Alberta (ESAA) Benvironmental Services Association of Alberta (ESAA) AGAT Laboratories is accredited to ISO/IEC 17025 by the Canadian Association for Laboratory Accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific tests listed on the scope of accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific tests listed on the scope of accreditation Inc. (CALA) for specific drinking water tests. Accreditations are location and parameter specific. A complete listing of parameters for each location is available from www.cala.ca and/or www.scc.ca. The tests in this report may not necessarily be included in the scope of accreditation. Measurement Uncertainty is not taken into consideration when stating conformity with a specified requirement.

Results relate only to the items tested. Results apply to samples as received. All reportable information as specified by ISO 17025:2017 is available from AGAT Laboratories upon request



AGAT WORK ORDER: 19T428150 PROJECT: TPB1881457.2000.1 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

SAMPLING SITE:

ATTENTION TO: Willie Kokotec

SAMPLED BY:

O. Reg. 153(511) - Metals & Inorganics (Soil)

, ,	SAMPLE DES		BH1-SS6	BH2-SS5
			Soil	Soil
			2010-01-04	2010-01-04
Unit	G/S	RDI	9836867	9836868
	13	0.8	<0.8	<0.8
µg/g	1.5	1	<0.0 5	7
µg/g	220	2	62	53
µg/g	220	2	02 -0 F	-0 F
µg/g	2.5	0.5	<0.5	<0.5
µg/g	36	5	<0	/
hð\ð	NA	0.10	0.16	0.38
µg/g	1.2	0.5	<0.5	<0.5
µg/g	70	2	14	10
µg/g	21	0.5	6.0	5.7
µg/g	92	1	34	28
µg/g	120	1	17	23
µg/g	2	0.5	0.6	0.9
µg/g	82	1	13	10
µg/g	1.5	0.4	<0.4	<0.4
µg/g	0.5	0.2	<0.2	<0.2
µg/g	1	0.4	<0.4	<0.4
µg/g	2.5	0.5	0.7	1.1
µg/g	86	1	20	18
µg/g	290	5	81	77
µg/g	0.66	0.2	<0.2	<0.2
hđ/đ	0.051	0.040	<0.040	<0.040
ha/a	0.27	0.10	<0.10	<0.10
mS/cm	0.57	0.005	1.12	1.19
NA	2.4	NA	4.48	3.86
pH Units		NA	7.36	7.38
	Unit µ9/9	SAMPLE DESI SAMPLE DESI DATE S Unit G / S µg/g 1.3 µg/g 1.3 µg/g 1.3 µg/g 2.0 µg/g 2.5 µg/g 2.5 µg/g 3.6 µg/g 1.2 µg/g 7.0 µg/g 2.1 µg/g 2.1 µg/g 2.1 µg/g 9.2 µg/g 1.2 µg/g 0.1 µg/g 2.1 µg/g 9.2 µg/g 1.2 µg/g 1.2 µg/g 1.2 µg/g 2.1 µg/g 2.1 µg/g 1.2 µg/g 3.6 µg/g 0.5 µg/g 2.5 µg/g 2.6 µg/g 0.66 µg/g 0.27 <	SAMPLE DESCRIPTION: SAMPLE TYPE: DATE SAMPLED: Unit G / S RDL µg/g 1.3 0.8 µg/g 1.8 1 µg/g 220 2 µg/g 2.5 0.5 µg/g 36 5 µg/g 1.2 0.5 µg/g 70 2 µg/g 21 0.5 µg/g 120 1 µg/g 120 1 µg/g 1.5 0.4 µg/g 0.5 0.2 µg/g 0.5 0.2 µg/g 1.5 0.4 µg/g 0.5 0.2 µg/g 1 0.4 µg/g 2.5 0.5 µg/g 0.66 0.2 µg/g 0.051 0.040 µg/g 0.27 0.10 mS/cm 0.57 0.005 NA	SAMPLE DESCRIPTION: BH1-SS6 SAMPLE TYPE: Soil DATE SAMPLED: 2019-01-04 Unit G / S RDL 9836867 µg/g 1.3 0.8 <0.8

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Table 1: Full Depth Background Site Condition Standards - Soil -Residential/Parkland/Institutional/Industrial/Commercial/Community Property Use

9836867-9836868
 EC & SAR were determined on the DI water extract obtained from the 2:1 leaching procedure (2 parts DI water:1 part soil). pH was determined on the 0.01M CaCl2 extract prepared at 2:1 ratio.

Analysis performed at AGAT Toronto (unless marked by *)

Certified By:



DATE REPORTED: 2019-01-21



Guideline Violation

AGAT WORK ORDER: 19T428150 PROJECT: TPB1881457.2000.1 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

ATTENTION TO: Willie Kokotec

SAMPLEID	SAMPLE TITLE	GUIDELINE	ANALYSIS PACKAGE	PARAMETER	UNIT	GUIDEVALUE	RESULT
9836867	BH1-SS6	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity	mS/cm	0.57	1.12
9836867	BH1-SS6	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio	NA	2.4	4.48
9836868	BH2-SS5	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Electrical Conductivity	mS/cm	0.57	1.19
9836868	BH2-SS5	ON T1 S RPI/ICC	O. Reg. 153(511) - Metals & Inorganics (Soil)	Sodium Adsorption Ratio	NA	2.4	3.86



Quality Assurance

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

SAMPLING SITE:

AGAT WORK ORDER: 19T428150 ATTENTION TO: Willie Kokotec

SAMPLED BY:

Soil Analysis

													r		
RPT Date: Jan 21, 2019	Date: Jan 21, 2019		C 1	UPLICAT	E		REFERE	NCE MA	TERIAL	METHOD	BLAN	(SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dun #2	RPD	Method Blank	Measured	Acce Lii	eptable nits	Recovery	Acce Lir	ptable nits	Recovery	Acce Lir	ptable nits
	Duton	ld	Dup "	Dup #2			Value	Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 153(511) - Metals & Inor	anics (Soil)		1			1	1			1			1		
Antimony	9836894		<0.8	<0.8	NA	< 0.8	97%	70%	130%	82%	80%	120%	93%	70%	130%
Arsenic	9836894		3	3	NA	< 1	114%	70%	130%	94%	80%	120%	98%	70%	130%
Barium	9836894		59	62	5.0%	< 2	105%	70%	130%	100%	80%	120%	103%	70%	130%
Bervllium	9836894		<0.5	< 0.5	NA	< 0.5	99%	70%	130%	98%	80%	120%	86%	70%	130%
Boron	9836894		<5	5	NA	< 5	70%	70%	130%	101%	80%	120%	86%	70%	130%
Boron (Hot Water Soluble)	9836894		<0.10	<0.10	NA	< 0.10	94%	60%	140%	98%	70%	130%	99%	60%	140%
Cadmium	9836894		<0.5	<0.5	NA	< 0.10	107%	70%	130%	97%	80%	120%	100%	70%	130%
Chromium	9836894		11	11	0.0%	< 2	93%	70%	130%	95%	80%	120%	106%	70%	130%
Cobalt	9836894		49	5.0	2.0%	< 0.5	96%	70%	130%	98%	80%	120%	100%	70%	130%
Copper	9836894		10	10	0.0%	< 1	100%	70%	130%	103%	80%	120%	99%	70%	130%
Lead	9836894		5	5	0.0%	<i>-</i> 1	110%	70%	130%	105%	80%	120%	101%	70%	130%
Molybdenum	9836894		<05	<05	NA	< 0.5	102%	70%	130%	102%	80%	120%	109%	70%	130%
Nickel	9836894		10	10	0.0%	< 1	99%	70%	130%	98%	80%	120%	101%	70%	130%
Selenium	9836894		<0.4	<0.4	NA	< 0.4	101%	70%	130%	102%	80%	120%	103%	70%	130%
Silver	9836894		<0.2	<0.4	NA	< 0.2	101%	70%	130%	92%	80%	120%	94%	70%	130%
Thallium	0926904		-0.4	-0.4	NIA	.0.4	059/	700/	1200/	00%	0.00/	1000/	069/	700/	1200/
Liranium	9030094		<0.4	<0.4		< 0.4	1000/	70%	120%	9976 1009/	00%	120%	90% 102%	70%	130%
Vapadium	9030094		<0.5 10	<0.5 10	E 49/	< 0.5	0.00/	70%	120%	070/	00%	120%	102%	70%	130%
Zino	9030094		10	19	0.4%	< 1	90%	70%	120%	97%	00% 00%	120%	101%	70%	120%
	9030094		20	-0.2	3.0%	< 0.2	1100/	70%	120%	101%	00%	120%	100%	70%	120%
	9030004		<0.2	<0.2	INA	< 0.2	110%	70%	130%	100%	00%	120%	103%	70%	130%
Cyanide	9838618		<0.040	<0.040	NA	< 0.040	92%	70%	130%	103%	80%	120%	91%	70%	130%
Mercury	9836894		<0.10	<0.10	NA	< 0.10	105%	70%	130%	103%	80%	120%	96%	70%	130%
Electrical Conductivity	9836354		0.541	0.549	1.5%	< 0.005	97%	90%	110%	NA			NA		
Sodium Adsorption Ratio	9832792		0.561	0.575	2.5%	NA	NA			NA			NA		
pH, 2:1 CaCl2 Extraction	9836864		7.46	7.49	0.4%	NA	101%	80%	120%	NA			NA		
O. Reg. 558 Metals and Inorgani	cs														
Arsenic Leachate	9836897		<0.010	<0.010	NA	< 0.010	103%	90%	110%	95%	80%	120%	98%	70%	130%
Barium Leachate	9836897		0.253	0.256	NA	< 0.100	103%	90%	110%	98%	80%	120%	103%	70%	130%
Boron Leachate	9836897		0.054	0.059	NA	< 0.050	99%	90%	110%	92%	80%	120%	89%	70%	130%
Cadmium Leachate	9836897		<0.010	<0.010	NA	< 0.010	105%	90%	110%	102%	80%	120%	105%	70%	130%
Chromium Leachate	9836897		<0.010	<0.010	NA	< 0.010	105%	90%	110%	99%	80%	120%	107%	70%	130%
Lead Leachate	9836897		1.09	1.11	1.8%	< 0.010	108%	90%	110%	93%	80%	120%	115%	70%	130%
Mercury Leachate	9836897		<0.01	<0.01	NA	< 0.01	103%	90%	110%	99%	80%	120%	99%	70%	130%
Selenium Leachate	9836897		<0.010	<0.010	NA	< 0.010	102%	90%	110%	96%	80%	120%	99%	70%	130%
Silver Leachate	9836897		<0.010	<0.010	NA	< 0.010	108%	90%	110%	95%	80%	120%	102%	70%	130%
Uranium Leachate	9836897		<0.050	<0.050	NA	< 0.050	107%	90%	110%	96%	80%	120%	103%	70%	130%
Fluoride Leachate	9836897		0.32	0.33	3.1%	< 0.05	100%	90%	110%	101%	90%	110%	102%	70%	130%
Cyanide Leachate	9836897		<0.05	<0.05	NA	< 0.05	92%	90%	110%	103%	90%	110%	116%	70%	130%
(Nitrate + Nitrite) as N Leachate	9836897		<0.70	<0.70	NA	< 0.70	99%	80%	120%	100%	80%	120%	105%	70%	130%

AGAT QUALITY ASSURANCE REPORT (V1)

Page 4 of 9

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Quality Assurance

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

SAMPLING SITE:

AGAT WORK ORDER: 19T428150

ATTENTION TO: Willie Kokotec

SAMPLED BY:

Soil Analysis (Continued)

RPT Date: Jan 21, 2019			0	UPLICAT	E		REFEREN	ICE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lir	ptable nits	Recoverv	Acce Lir	ptable nits	Recoverv	Acce Lir	ptable nits
		Id	•				value	Lower	Upper		Lower	Upper		Lower	Upper

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL





AGAT QUALITY ASSURANCE REPORT (V1)

Page 5 of 9

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Quality Assurance

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

SAMPLING SITE:

AGAT WORK ORDER: 19T428150 ATTENTION TO: Willie Kokotec SAMPLED BY:

Trace Organics Analysis

					-		-						· · · · · · · · · · · · · · · · · · ·		
RPT Date: Jan 21, 2019	RPT Date: Jan 21, 2019		C	UPLICAT	E		REFEREN	NCE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SP	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lir	ptable nits	Recovery	Acce Lir	ptable nits	Recovery	Acce	ptable nits
		iu iu					value	Lower	Upper	_	Lower	Upper		Lower	Upper
O. Reg. 558 - VOCs															
Vinyl Chloride	9836869	9836869	< 0.030	< 0.030	NA	< 0.030	100%	60%	140%	116%	60%	140%	NA	60%	140%
1,1 Dichloroethene	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	77%	70%	130%	101%	70%	130%	NA	60%	140%
Dichloromethane	9836869	9836869	< 0.030	< 0.030	NA	< 0.030	84%	70%	130%	98%	70%	130%	NA	60%	140%
Methyl Ethyl Ketone	9836869	9836869	< 0.090	< 0.090	NA	< 0.090	106%	70%	130%	83%	70%	130%	NA	60%	140%
Chloroform	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	87%	70%	130%	79%	70%	130%	NA	60%	140%
1,2-Dichloroethane	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	89%	70%	130%	85%	70%	130%	NA	60%	140%
Carbon Tetrachloride	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	85%	70%	130%	101%	70%	130%	NA	60%	140%
Benzene	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	80%	70%	130%	85%	70%	130%	NA	60%	140%
Trichloroethene	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	87%	70%	130%	89%	70%	130%	NA	60%	140%
Tetrachloroethene	9836869	9836869	< 0.050	< 0.050	NA	< 0.050	75%	70%	130%	116%	70%	130%	NA	60%	140%
Chlorobenzene	9836869	9836869	< 0.010	< 0.010	NA	< 0.010	80%	70%	130%	108%	70%	130%	NA	60%	140%
1,2-Dichlorobenzene	9836869	9836869	< 0.010	< 0.010	NA	< 0.010	83%	70%	130%	101%	70%	130%	NA	60%	140%
1,4-Dichlorobenzene	9836869	9836869	< 0.010	< 0.010	NA	< 0.010	83%	70%	130%	107%	70%	130%	NA	60%	140%
O. Reg. 558 - Benzo(a) pyrene			0.004	0.004		0.004	000/	700/	4000/	1000/	700/	4000/	N 14	700/	4000/
Benzo(a)pyrene	9836869	9836869	< 0.001	< 0.001	NA	< 0.001	93%	70%	130%	103%	70%	130%	NA	70%	130%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

wg

Page 6 of 9

AGAT QUALITY ASSURANCE REPORT (V1)

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Method Summary

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

AGAT WORK ORDER: 19T428150 **ATTENTION TO: Willie Kokotec**

SAMPLING SITE:		SAMPLED BY:	
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Soil Analysis		•	
Antimony	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Arsenic	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Barium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Beryllium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Boron (Hot Water Soluble)	MET-93-6104	EPA SW 846 6010C; MSA, Part 3, Ch.21	ICP/OES
Cadmium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Cobalt	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Copper	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Lead	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Molybdenum	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Nickel	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Selenium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Silver	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Thallium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Uranium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Vanadium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Zinc	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Chromium VI	INOR-93-6029	SM 3500 B; MSA Part 3, Ch. 25	SPECTROPHOTOMETER
Cyanide	INOR-93-6052	MOE CN-3015 & E 3009 A;SM 4500 CN	TECHNICON AUTO ANALYZER
Mercury	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS
Electrical Conductivity	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER
Sodium Adsorption Ratio	INOR-93-6007	McKeague 4.12 & 3.26 & EPA SW-846 6010C	ICP/OES
pH, 2:1 CaCl2 Extraction	INOR-93-6031	MSA part 3 & SM 4500-H+ B	PH METER
Arsenic Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Barium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Boron Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Cadmium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Chromium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Lead Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Mercury Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Selenium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Silver Leachate	MET -93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Uranium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS
Fluoride Leachate	INOR-93-6018	EPA SW-846-1311 & SM4500-F- C	ION SELECTIVE ELECTRODE
Cyanide Leachate	INOR-93-6052	EPA SW-846-1311 & MOE 3015 & SM 4500 CN- I	TECHNICON AUTO ANALYZER
(Nitrate + Nitrite) as N Leachate	INOR-93-6053	EPA SW 846-1311 & SM 4500 - NO3- I	LACHAT FIA



Method Summary

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

AGAT WORK ORDER: 19T428150 ATTENTION TO: Willie Kokotec

SAMPLING SITE:		SAMPLED BY:	
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Trace Organics Analysis	I		1
Benzo(a)pyrene	ORG-91-5105	EPA SW846 3540 & 8270	GC/MS
Vinyl Chloride	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
1,1 Dichloroethene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Dichloromethane	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Methyl Ethyl Ketone	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Chloroform	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
1,2-Dichloroethane	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Carbon Tetrachloride	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Benzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Trichloroethene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Tetrachloroethene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Chlorobenzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
1,2-Dichlorobenzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
1,4-Dichlorobenzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS
Toluene-d8	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS

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Reports to be sent to:	Willie Kakator	a Hax:	de con	1		Agriculture	Region		_		bjectives	(PWQC	y)	Ru	sh 1/	AI (Rus	h Surch	arges Ap	piy)				
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CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS 3450 HARVESTER ROAD, SUITE 100 BURLINGTON, ON L7N 3W5 (905) 335-2353

ATTENTION TO: Willie Kokotec

PROJECT: TPB1881457.2000.1

AGAT WORK ORDER: 19T428150

SOIL ANALYSIS REVIEWED BY: Amanjot Bhela, Inorganic Supervisor

TRACE ORGANICS REVIEWED BY: Oksana Gushyla, Trace Organics Lab Supervisor

DATE REPORTED: Jan 21, 2019

PAGES (INCLUDING COVER): 10

VERSION*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

<u>"NOTES</u>

All samples will be disposed of within 30 days following analysis. Please contact the lab if you require additional sample storage time.

 AGAT Laboratories (V1)
 Page 1 of 10

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Results relate only to the items tested. Results apply to samples as received. All reportable information as specified by ISO 17025:2017 is available from AGAT Laboratories upon request



AGAT WORK ORDER: 19T428150 PROJECT: TPB1881457.2000.1 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

SAMPLING SITE:

ATTENTION TO: Willie Kokotec

SAMPLED BY:

				O. Reg. 5	58 Metals and Inorganics
DATE RECEIVED: 2019-01-15					DATE REPORTED: 2019-01-21
	S	AMPLE DES	CRIPTION:	TCLP	
		SAM	PLE TYPE:	Soil	
		DATES	SAMPLED:	2019-01-04	
Parameter	Unit	G/S	RDL	9836869	
Arsenic Leachate	mg/L	2.5	0.010	<0.010	
Barium Leachate	mg/L	100	0.100	0.239	
Boron Leachate	mg/L	500	0.050	<0.050	
Cadmium Leachate	mg/L	0.5	0.010	<0.010	
Chromium Leachate	mg/L	5	0.010	<0.010	
Lead Leachate	mg/L	5	0.010	<0.010	
Mercury Leachate	mg/L	0.1	0.01	<0.01	
Selenium Leachate	mg/L	1	0.010	<0.010	
Silver Leachate	mg/L	5	0.010	<0.010	
Uranium Leachate	mg/L	10	0.050	<0.050	
Fluoride Leachate	mg/L	150	0.05	0.23	
Cyanide Leachate	mg/L	20	0.05	<0.05	
(Nitrate + Nitrite) as N Leachate	mg/L	1000	0.70	<0.70	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to O. Reg. 558 - Schedule IV Leachate Quality Criteria

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation. Analysis performed at AGAT Toronto (unless marked by *)

Amanjot Bhelle

Certified By:



AGAT WORK ORDER: 19T428150 PROJECT: TPB1881457.2000.1

5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.agatlabs.com

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

SAMPLING SITE:

ATTENTION TO: Willie Kokotec

SAMPLED BY:

				O. Reg	i. 558 - Benzo(a) pyrene
DATE RECEIVED: 2019-01-15					DATE REPORTED: 2019-01-21
	Ś	SAMPLE DES	CRIPTION:	TCLP	
		SAM	PLE TYPE:	Soil	
		DATE	SAMPLED:	2019-01-04	
Parameter	Unit	G/S	RDL	9836869	
Benzo(a)pyrene	mg/L	0.001	0.001	<0.001	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to O. Reg. 558 - Schedule IV Leachate Quality Criteria Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

9836869 The sample was leached according to Regulation 558 protocol. Analysis was performed on the leachate.

Analysis performed at AGAT Toronto (unless marked by *)

Certified By:



AGAT WORK ORDER: 19T428150 PROJECT: TPB1881457.2000.1 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

SAMPLING SITE:

ATTENTION TO: Willie Kokotec

SAMPLED BY:

O. Reg. 558 - VOCs DATE RECEIVED: 2019-01-15 **DATE REPORTED: 2019-01-21** TCLP SAMPLE DESCRIPTION: SAMPLE TYPE: Soil DATE SAMPLED: 2019-01-04 G/S RDL 9836869 Parameter Unit Vinyl Chloride mg/L 0.2 0.030 < 0.030 1,1 Dichloroethene mg/L 1.4 0.020 < 0.020 Dichloromethane mg/L 5.0 0.030 < 0.030 200 Methyl Ethyl Ketone mg/L 0.090 < 0.090 Chloroform mg/L 10.0 0.020 <0.020 1.2-Dichloroethane mg/L 0.5 0.020 < 0.020 Carbon Tetrachloride mg/L 0.5 0.020 < 0.020 Benzene mg/L 0.5 0.020 < 0.020 5.0 0.020 <0.020 Trichloroethene mg/L Tetrachloroethene mg/L 3.0 0.050 < 0.050 Chlorobenzene mg/L 8.0 0.010 < 0.010 20.0 0.010 < 0.010 1,2-Dichlorobenzene mg/L 1,4-Dichlorobenzene mg/L 0.5 0.010 <0.010 Unit Surrogate Acceptable Limits Toluene-d8 % Recovery 60-130 76

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to O. Reg. 558 - Schedule IV Leachate Quality Criteria

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

9836869 Sample was prepared using Regulation 558 protocol and a zero headspace extractor.

Analysis performed at AGAT Toronto (unless marked by *)

Certified By:



Page 5 of 10

Quality Assurance

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

SAMPLING SITE:

AGAT WORK ORDER: 19T428150

ATTENTION TO: Willie Kokotec SAMPLED BY:

Soil Analysis DUPLICATE REFERENCE MATERIAL METHOD BLANK SPIKE RPT Date: Jan 21, 2019 MATRIX SPIKE Method Acceptable Acceptable Acceptable Sample Measured Blank Limits Limits Limits PARAMETER Batch Dup #1 Dup #2 RPD Recovery Recovery Value Id Lower Upper Lower Upper Lower Upper O. Reg. 153(511) - Metals & Inorganics (Soil) 70% 93% 130% Antimony 9836894 NA 97% 130% 82% 80% 120% 70% <0.8 <0.8 < 0.8 130% Arsenic 9836894 3 3 NA < 1 114% 70% 130% 94% 80% 120% 98% 70% Barium 9836894 59 62 5.0% < 2 105% 70% 130% 100% 80% 120% 103% 70% 130% Beryllium 9836894 <0.5 <0.5 NA < 0.5 99% 70% 130% 98% 80% 120% 86% 70% 130% 9836894 5 NA 70% 70% 130% 101% 80% 120% 86% 70% 130% Boron <5 < 5 Boron (Hot Water Soluble) <0.10 <0.10 < 0.10 94% 60% 140% 98% 130% 99% 60% 140% 9836894 NA 70% Cadmium 9836894 <0.5 <0.5 NA < 0.5 107% 70% 130% 97% 80% 120% 100% 70% 130% Chromium 9836894 11 11 0.0% < 2 93% 70% 130% 95% 80% 120% 106% 70% 130% Cobalt 9836894 4.9 5.0 2.0% < 0.5 96% 70% 130% 98% 80% 120% 100% 70% 130% 70% 103% 9836894 10 10 0.0% 100% 130% 80% 120% 99% 70% 130% Copper < 1 5 0.0% 105% 101% 130% Lead 9836894 5 < 1 110% 70% 130% 80% 120% 70% < 0.5 < 0.5 102% 70% 102% 109% 130% Molybdenum 9836894 < 0.5 NA 130% 80% 120% 70% 130% Nickel 9836894 10 10 0.0% < 1 99% 70% 130% 98% 80% 120% 101% 70% Selenium 9836894 <0.4 <0.4 NA < 0.4 101% 70% 130% 102% 80% 120% 103% 70% 130% Silver 9836894 <0.2 <0.2 NA < 0.2 101% 70% 130% 92% 80% 120% 94% 70% 130% Thallium 9836894 <0.4 <0.4 NA < 0.4 95% 70% 130% 99% 80% 120% 96% 70% 130% Uranium 9836894 < 0.5 < 0.5 NA < 0.5 108% 70% 130% 100% 80% 120% 102% 70% 130% Vanadium 9836894 18 19 5 4% < 1 98% 70% 130% 97% 80% 120% 101% 70% 130% Zinc 9836894 26 27 3.8% < 5 103% 70% 130% 101% 80% 120% 106% 70% 130% Chromium VI 9836864 <0.2 <0.2 NA < 0.2 110% 70% 130% 100% 80% 120% 103% 130% 70% Cyanide 9838618 < 0.040 < 0.040 < 0.040 92% 70% 130% 103% 91% 70% 130% NA 80% 120% 103% <0.10 <0.10 NA 105% 70% 130% 80% 120% 96% 70% 130% Mercurv 9836894 < 0.10**Electrical Conductivity** 97% 0.541 0.549 90% 9836354 1.5% < 0.005110% NΑ NA Sodium Adsorption Ratio 0.561 0.575 NΑ NA 9832792 2.5% NA NA pH, 2:1 CaCl2 Extraction 9836864 7.46 7.49 0.4% NA 101% 80% 120% NA NA O. Reg. 558 Metals and Inorganics Arsenic Leachate 9836897 <0.010 <0.010 NA < 0.010 103% 90% 110% 95% 80% 120% 98% 70% 130% **Barium Leachate** 9836897 0.253 0.256 NA < 0.100 103% 90% 110% 98% 80% 120% 103% 70% 130% < 0.050 90% 92% Boron Leachate 9836897 0.054 0.059 NA 99% 110% 80% 120% 89% 70% 130% Cadmium Leachate 9836897 <0.010 <0.010 NA < 0.010 105% 90% 110% 102% 80% 120% 105% 70% 130% Chromium Leachate 9836897 <0.010 <0.010 NA < 0.010 105% 90% 110% 99% 80% 120% 107% 70% 130% Lead Leachate 9836897 1.09 1.8% < 0.010 108% 93% 120% 130% 90% 110% 80% 115% 70% 1.11 130% 9836897 < 0.01 < 0.01 103% 90% 99% 99% Mercury Leachate < 0.01 NA 110% 80% 120% 70% 9836897 NA 102% 90% 96% 99% 130% Selenium Leachate < 0.010 < 0.010 < 0.010110% 80% 120% 70% 108% 9836897 < 0.010 < 0.010 < 0.010 90% 95% 102% 70% 130% Silver Leachate NA 110% 80% 120% 130% Uranium Leachate 9836897 < 0.050 < 0.050 NA < 0.050 107% 90% 110% 96% 80% 120% 103% 70% Fluoride Leachate 9836897 0.32 0.33 3.1% < 0.05 100% 90% 110% 101% 90% 110% 102% 70% 130% Cyanide Leachate 9836897 < 0.05 < 0.05 NA < 0.05 92% 90% 110% 103% 90% 110% 116% 70% 130% 80% (Nitrate + Nitrite) as N Leachate 9836897 <0.70 <0.70 NA < 0.70 99% 80% 120% 100% 120% 105% 70% 130%

AGAT QUALITY ASSURANCE REPORT (V1)

AGAT Laboratories is accredited to ISO/IEC 17025 by the Canadian Association for Laboratory Accreditation Inc. (CALA) and/or Standards Council of Canada (SCC) for specific tests listed on the scope of accreditation. AGAT Laboratories (Mississauga) is also accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA) for specific drinking water tests. Accreditations are location and parameter specific. A complete listing of parameters for each location is available from www.cala.ca and/or www.scc.ca. The tests in this report may not necessarily be included in the scope of accreditation. RPDs calculated using raw data. The RPD may not be reflective of duplicate values shown, due to rounding of final results.



Quality Assurance

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

SAMPLING SITE:

AGAT WORK ORDER: 19T428150

ATTENTION TO: Willie Kokotec

SAMPLED BY:

Soil Analysis (Continued)

RPT Date: Jan 21, 2019			C	UPLICAT	E		REFEREN	ICE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lin	ptable nits	Recoverv	Acce Lin	ptable nits	Recoverv	Acce Lin	ptable nits
		Id					value	Lower	Upper		Lower	Upper		Lower	Upper

Comments: NA signifies Not Applicable.

Duplicate Qualifier: As the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL





AGAT QUALITY ASSURANCE REPORT (V1)

Page 6 of 10

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Quality Assurance

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

SAMPLING SITE:

AGAT WORK ORDER: 19T428150 ATTENTION TO: Willie Kokotec SAMPLED BY:

Trace Organics Analysis

			1		-	1	-								
RPT Date: Jan 21, 2019	_			UPLICAT	E		REFEREN	NCE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lir	eptable nits	Recovery	Acce Lir	ptable nits	Recovery	Acce Lir	ptable nits
		iu		-			value	Lower	Upper		Lower	Upper		Lower	Upper
O. Reg. 558 - VOCs															
Vinyl Chloride	9836869	9836869	< 0.030	< 0.030	NA	< 0.030	100%	60%	140%	116%	60%	140%	NA	60%	140%
1,1 Dichloroethene	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	77%	70%	130%	101%	70%	130%	NA	60%	140%
Dichloromethane	9836869	9836869	< 0.030	< 0.030	NA	< 0.030	84%	70%	130%	98%	70%	130%	NA	60%	140%
Methyl Ethyl Ketone	9836869	9836869	< 0.090	< 0.090	NA	< 0.090	106%	70%	130%	83%	70%	130%	NA	60%	140%
Chloroform	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	87%	70%	130%	79%	70%	130%	NA	60%	140%
1,2-Dichloroethane	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	89%	70%	130%	85%	70%	130%	NA	60%	140%
Carbon Tetrachloride	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	85%	70%	130%	101%	70%	130%	NA	60%	140%
Benzene	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	80%	70%	130%	85%	70%	130%	NA	60%	140%
Trichloroethene	9836869	9836869	< 0.020	< 0.020	NA	< 0.020	87%	70%	130%	89%	70%	130%	NA	60%	140%
Tetrachloroethene	9836869	9836869	< 0.050	< 0.050	NA	< 0.050	75%	70%	130%	116%	70%	130%	NA	60%	140%
Chlorobenzene	9836869	9836869	< 0.010	< 0.010	NA	< 0.010	80%	70%	130%	108%	70%	130%	NA	60%	140%
1,2-Dichlorobenzene	9836869	9836869	< 0.010	< 0.010	NA	< 0.010	83%	70%	130%	101%	70%	130%	NA	60%	140%
1,4-Dichlorobenzene	9836869	9836869	< 0.010	< 0.010	NA	< 0.010	83%	70%	130%	107%	70%	130%	NA	60%	140%
O. Reg. 558 - Benzo(a) pyrene															
Benzo(a)pyrene	9836869	9836869	< 0.001	< 0.001	NA	< 0.001	93%	70%	130%	103%	70%	130%	NA	70%	130%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

wg

Page 7 of 10

AGAT QUALITY ASSURANCE REPORT (V1)

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Method Summary

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

AGAT WORK ORDER: 19T428150

SAMDI INC SITE	
SAMPLING SHE.	

ATTENTION TO: Willie Kokotec SAMPLED BY:

PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE						
Soil Analysis		1							
Antimony	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Arsenic	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Barium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Beryllium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Boron	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Boron (Hot Water Soluble)	MET-93-6104	EPA SW 846 6010C; MSA, Part 3, Ch.21	ICP/OES						
Cadmium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Chromium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Cobalt	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Copper	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Lead	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Molybdenum	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Nickel	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Selenium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Silver	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Thallium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Uranium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Vanadium	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Zinc	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Chromium VI	INOR-93-6029	SM 3500 B; MSA Part 3, Ch. 25	SPECTROPHOTOMETER						
Cyanide	INOR-93-6052	MOE CN-3015 & E 3009 A;SM 4500 CN	TECHNICON AUTO ANALYZER						
Mercury	MET-93-6103	EPA SW-846 3050B & 6020A	ICP-MS						
Electrical Conductivity	INOR-93-6036	McKeague 4.12, SM 2510 B	EC METER						
Sodium Adsorption Ratio	INOR-93-6007	McKeague 4.12 & 3.26 & EPA SW-846 6010C	CP/OES						
pH, 2:1 CaCl2 Extraction	INOR-93-6031	MSA part 3 & SM 4500-H+ B	PH METER						
Arsenic Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Barium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Boron Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Cadmium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Chromium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Lead Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Mercury Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Selenium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Silver Leachate	MET -93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Uranium Leachate	MET-93-6103	EPA SW-846 1311 & 3010A & 6020A	ICP-MS						
Fluoride Leachate	INOR-93-6018	EPA SW-846-1311 & SM4500-F- C	ION SELECTIVE ELECTRODE						
Cyanide Leachate	INOR-93-6052	EPA SW-846-1311 & MOE 3015 & SM 4500 CN- I	TECHNICON AUTO ANALYZER						
(Nitrate + Nitrite) as N Leachate	INOR-93-6053	EPA SW 846-1311 & SM 4500 - NO3- I	LACHAT FIA						



Method Summary

CLIENT NAME: WOOD ENVIRONMENT & INFRASTRUCTURE SOLUTIONS

PROJECT: TPB1881457.2000.1

AGAT WORK ORDER: 19T428150 ATTENTION TO: Willie Kokotec

SAMPLING SITE:	AMPLING SITE: SAMPLED BY:									
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE							
Trace Organics Analysis	I									
Benzo(a)pyrene	ORG-91-5105	EPA SW846 3540 & 8270	GC/MS							
Vinyl Chloride	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
1,1 Dichloroethene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Dichloromethane	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Methyl Ethyl Ketone	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Chloroform	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
1,2-Dichloroethane	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Carbon Tetrachloride	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Benzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Trichloroethene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Tetrachloroethene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Chlorobenzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
1,2-Dichlorobenzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
1,4-Dichlorobenzene	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							
Toluene-d8	VOL-91-5001	EPA SW-846 5230B & 8260	(P&T)GC/MS							

									- Laboratory Use Only													
E G G G Laboratories						5835 Coopers Avenue Mississauga, Ontario L4Z 1Y2 Ph: 905.712.5100 Fax: 905.712.5122						Work Order #: 197428150										
Chain of Custody Record If this is a Drinking Water sample, please				lease use	webearth.agatiabs.com					Cooler Quantity:												
Report Information:				Regulatory Requirements: 🔀 No Regulatory Requirement						nent	t Custody Seal Intact:											
Contact: Address: Address: Burlington, ON L7N 3W5 Phone: Reports to be sent to: 1. Email: Willie. Kokotec @uodplc. Com			[[] [] s	Regulation 153/04 Sewer Use Regulation 5 Table Ind/come Sanitary CCME Ind/Com Sanitary CCME Agriculture Storm Prov. Water Soil Texture (Check one) Region Other				558 r Quali (PWQC	y))	Notes: Turnaround Time (TAT) Required: Regular TAT 5 to 7 Business Da Rush TAT (Rush Surcharges Apply) 3 Business 2 Business						>d: s Days	lext Bus	iness				
2. Email: Homas. Howat Quadple. (Om			Fine MISA			Indicate One					OR Date Required (Rush Surcharges May Apply):											
Project Information: Project: TPB 881475.2000. (Site Location: Reid Sclerood, Milton, ON Sampled By: Thomas Hornat			_	Is this submission for a Report Guideline on Record of Site Condition? Certificate of Analysis Yes No Yes No					Please provide prior notification for rush TAT *TAT is exclusive of weekends and statutory holidays For 'Same Day' analysis, please contact your AGAT CPM													
AGAT Quote #: Standlerch Zoisk FXA PO: Please note: If quotation number is not provided, client will be billed full price for analysis. Invoice Information: Bill To Same: Yes INO I Company: Wood Contact: Willle, Kokotec Address: 3450 Harvester Rd, Suite 100, Burlington ON Email: Willie, Kokotec Quoodplc. Com				Sample Matrix LegendBBiotaGWGround WaterOOilPPaintSSoilSDSedimentSWSurface Water		Field Filtered - Metals, Hg, CrVI	and Inorganics	tals — 153 Metals (excl. Hydrides) O e Metals — 153 Metals (Incl. Hydrides)	□BHWS コCi コCN コEC コFOC コHg JSAR	tals Scan	tion/Custom Metals ts: □TP □NH ₃ □TKN □NO □NO +NO	s: Dvoc DBTEX DTHM	1-F4		Total Droclors	chlorine Pesticides						
Sample Identification	Date Sampled	Time Sampled	# of Containers	Sample Matrix	Commen Special Instru	ts/ uctions	Y/N	Metals		ORPs: DCr.	Full Me	Regula Nutrier	Volatile	PHCs F	PAHs	PCBs: [Organo	Sewer			The second	
BHI-556 BH2-555 TCLP TCLP TCLP	Jan 4 11 11 11	9:00 10:30 12:00 12:00 12:00		55555				XX					8. 8. 10. 11. 11. 11. 11. 11. 11. 11. 11. 11									
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APPENDIX D REPORT LIMITATIONS

Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you - assumedly a client representative - interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled.* No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnicalengineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full*.

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be*, and, in general, *if you are the least bit uncertain* about the continued reliability of this report, contact your geotechnical engineer before applying it. A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmationdependent recommendations if you fail to retain that engineer to perform construction observation*.

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnicalengineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only.* To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnicalengineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old.*

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration*. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not buildingenvelope or mold specialists*.



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Limitations to Geotechnical Reports

The work performed in the preparation of this report and the conclusions presented herein are subject to the following:

- a) The contract between Wood and the Client, including any subsequent written amendment or Change Order dully signed by the parties (hereinafter together referred as the "Contract");
- b) Any and all time, budgetary, access and/or site disturbance, risk management preferences, constraints or restrictions as described in the contract, in this report, or in any subsequent communication sent by Wood to the Client in connection to the Contract; and
- c) The limitations stated herein.
- 2. **Standard of care:** Wood has prepared this report in a manner consistent with the level of skill and are ordinarily exercised by reputable members of Wood's profession, practicing in the same or similar locality at the time of performance, and subject to the time limits and physical constraints applicable to the scope of work, and terms and conditions for this assignment. No other warranty, guaranty, or representation, expressed or implied, is made or intended in this report, or in any other communication (oral or written) related to this project. The same are specifically disclaimed, including the implied warranties of merchantability and fitness for a particular purpose.
- 3. Limited locations: The information contained in this report is restricted to the site and structures evaluated by Wood and to the topics specifically discussed in it, and is not applicable to any other aspects, areas or locations.
- 4. **Information utilized:** The information, conclusions and estimates contained in this report are based exclusively on: i) information available at the time of preparation, ii) the accuracy and completeness of data supplied by the Client or by third parties as instructed by the Client, and iii) the assumptions, conditions and qualifications/limitations set forth in this report.
- 5. Accuracy of information: No attempt has been made to verify the accuracy of any information provided by the Client or third parties, except as specifically stated in this report (hereinafter "Supplied Data"). Wood cannot be held responsible for any loss or damage, of either contractual or extra-contractual nature, resulting from conclusions that are based upon reliance on the Supplied Data.
- 6. **Report interpretation:** This report must be read and interpreted in its entirety, as some sections could be inaccurately interpreted when taken individually or out-of-context. The contents of this report are based upon the conditions known and information provided as of the date of preparation. The text of the final version of this report supersedes any other previous versions produced by Wood.
- 7. **No legal representations:** Wood makes no representations whatsoever concerning the legal significance of its findings, or as to other legal matters touched on in this report, including but not limited to, ownership of any property, or the application of any law to the facts set forth herein. With respect to regulatory compliance issues, regulatory statutes are subject to interpretation and change. Such interpretations and regulatory changes should be reviewed with legal counsel.
- 8. **Decrease in property value:** Wood shall not be responsible for any decrease, real or perceived, of the property or site's value or failure to complete a transaction, as a consequence of the information contained in this report.
- 9. No third party reliance: This report is for the sole use of the party to whom it is addressed unless expressly stated otherwise in the report or Contract. Any use or reproduction which any third party makes of the report, in whole or in part, or any reliance thereon or decisions made based on any information or conclusions in the report is the sole responsibility of such third party. Wood does not represent or warrant the accuracy, completeness, merchantability, fitness for purpose or usefulness of this document, or any information contained in this document, for use or consideration by any third party. Wood accepts no responsibility whatsoever for damages or loss of any nature or kind suffered by any such third party as a result of actions taken or not taken or decisions made in reliance on this report or anything set out therein. including without limitation, any indirect, special, incidental, punitive or consequential loss, liability or damage of any kind.

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- 10. Assumptions: Where design recommendations are given in this report, they apply only if the project contemplated by the Client is constructed substantially in accordance with the details stated in this report. It is the sole responsibility of the Client to provide to Wood changes made in the project, including but not limited to, details in the design, conditions, engineering or construction that could in any manner whatsoever impact the validity of the recommendations made in the report. Wood shall be entitled to additional compensation from Client to review and assess the effect of such changes to the project.
- 11. **Time dependence**: If the project contemplated by the Client is not undertaken within a period of 18 months following the submission of this report, or within the time frame understood by Wood to be contemplated by the Client at the commencement of Wood's assignment, and/or, if any changes are made, for example, to the elevation, design or nature of any development on the site, its size and configuration, the location of any development on the site and its orientation, the use of the site, performance criteria and the location of any physical infrastructure, the conclusions and recommendations presented herein should not be considered valid unless the impact of the said changes is evaluated by Wood, and the conclusions of the report are amended or are validated in writing accordingly.

Advancements in the practice of geotechnical engineering, engineering geology and hydrogeology and changes in applicable regulations, standards, codes or criteria could impact the contents of the report, in which case, a supplementary report may be required. The requirements for such a review remain the sole responsibility of the Client or their agents.

Wood will not be liable to update or revise the report to take into account any events or emergent circumstances or facts occurring or becoming apparent after the date of the report.

- 12. Limitations of visual inspections: Where conclusions and recommendations are given based on a visual inspection conducted by Wood, they relate only to the natural or man-made structures, slopes, etc. inspected at the time the site visit was performed. These conclusions cannot and are not extended to include those portions of the site or structures, which were not reasonably available, in Wood's opinion, for direct observation.
- 13. Limitations of site investigations: Site exploration identifies specific subsurface conditions only at those points from which samples have been taken and only at the time of the site investigation. Site investigation programs are a professional estimate of the scope of investigation required to provide a general profile of subsurface conditions.

The data derived from the site investigation program and subsequent laboratory testing are interpreted by trained personnel and extrapolated across the site to form an inferred geological representation and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. Despite this investigation, conditions between and beyond the borehole/test hole locations may differ from those encountered at the borehole/test hole locations and the actual conditions at the site might differ from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies.

Final sub-surface/bore/profile logs are developed by geotechnical engineers based upon their interpretation of field logs and laboratory evaluation of field samples. Customarily, only the final bore/profile logs are included in geotechnical engineering reports.

Bedrock, soil properties and groundwater conditions can be significantly altered by environmental remediation and/or construction activities such as the use of heavy equipment or machinery, excavation, blasting, pile-driving or draining or other activities conducted either directly on site or on adjacent terrain. These properties can also be indirectly affected by exposure to unfavorable natural events or weather conditions, including freezing, drought, precipitation and snowmelt.

During construction, excavation is frequently undertaken which exposes the actual subsurface and groundwater conditions between and beyond the test locations, which may differ from those encountered at the test locations. It is recommended practice that Wood be retained during construction to confirm that the subsurface conditions throughout the site do not deviate materially from those encountered at the test locations, that construction work has no negative impact on the geotechnical aspects of the design, to adjust recommendations in accordance with conditions as additional site information is gained and to deal quickly with geotechnical considerations if they arise.



Interpretations and recommendations presented herein may not be valid if an adequate level of review or inspection by Wood is not provided during construction.

14. Factors that may affect construction methods, costs and scheduling: The performance of rock and soil materials during construction is greatly influenced by the means and methods of construction. Where comments are made relating to possible methods of construction, construction costs, construction techniques, sequencing, equipment or scheduling, they are intended only for the guidance of the project design professionals, and those responsible for construction monitoring. The number of test holes may not be sufficient to determine the local underground conditions between test locations that may affect construction costs, construction techniques, sequencing, equipment, scheduling, operational planning, etc.

Any contractors bidding on or undertaking the works should draw their own conclusions as to how the subsurface and groundwater conditions may affect their work, based on their own investigations and interpretations of the factual soil data, groundwater observations, and other factual information.

- 15. **Groundwater and Dewatering**: Wood will accept no responsibility for the effects of drainage and/or dewatering measures if Wood has not been specifically consulted and involved in the design and monitoring of the drainage and/or dewatering system.
- 16. Environmental and Hazardous Materials Aspects: Unless otherwise stated, the information contained in this report in no way reflects on the environmental aspects of this project, since this aspect is beyond the Scope of Work and the Contract. Unless expressly included in the Scope of Work, this report specifically excludes the identification or interpretation of environmental conditions such as contamination, hazardous materials, wild life conditions, rare plants or archeology conditions that may affect use or design at the site. This report specifically excludes the investigation, detection, prevention or assessment of conditions that can contribute to moisture, mould or other microbial contaminant growth and/or other moisture related deterioration, such as corrosion, decay, rot in buildings or their surroundings. Any statements in this report or on the boring logs regarding odours, colours, and unusual or suspicious items or conditions are strictly for informational purposes
- 17. **Sample Disposal**: Wood will dispose of all uncontaminated soil and rock samples after 30 days following the release of the final geotechnical report. Should the Client request that the samples be retained for a longer time, the Client will be billed for such storage at an agreed upon rate. Contaminated samples of soil, rock or groundwater are the property of the Client, and the Client will be responsible for the proper disposal of these samples, unless previously arranged for with Wood or a third party.

Wood Environment & Infrastructure Solutions, a Division of Wood Canada Limited