

# HYDROGEOLOGICAL ASSESSMENT PROPOSED NEW LONG TERM CARE BUILDING 6360 REGIONAL ROAD 25 MILTON, ONTARIO

Prepared For:	Thomas Robert Colbeck
	7050 Appleby Line
	Milton, Ontario
	L9E 0M5

Attention: Mr. Rob Colbeck

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	Terraprobe Inc.			
Greater Toronto	Hamilton – Niagara	Central Ontario	1	
11 Indell Lane	903 Barton Street, Unit 22	220 Bayview Drive, Unit 25		
Brampton, Ontario L6T 3Y3	Stoney Creek, Ontario L8E	Barrie, Ontario L4N 4Y8	S	
(905) 796-2650 Fax: 796-2250	(905) 643-7560 Fax: 643-7559	(705) 739-8355 Fax: 739-8369	(	
	www.terra	aprobe.ca		

#### Northern Ontario

1012 Kelly Lake Rd., Unit 1 **Sudbury**, Ontario P3E 5P4 (705) 670-0460 Fax: 670-0558

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# 1.0 INTRODUCTION

Terraprobe Inc. was retained by Thomas Robert Colbeck to conduct a hydrogeological assessment for the property with municipal address 6360 Regional Road 25, in Town of Milton, Ontario hereinafter referred to as the Property or Site. The Property is located southwest quadrant of the intersection of Regional Road 25 and Louis St Laurent Avenue. The general location of the Site is shown on **Figure 1**.

Based on the request for quotation, it is understood that the proposed development will include the construction of an 8 storey Long Term Care Home structure with the potential of one level of underground parking and services that would cover most of the site, parking areas and driveways/access routes. The purpose of this report is to assess local and regional hydrogeological conditions and the potential impacts of the proposed development on the groundwater system.

# 2.0 SCOPE OF WORK

The scope of work for the study consisted of the following:

- **<u>Review of available background information</u>:** Available background information for the site and the project was reviewed. This included the results of geotechnical and environmental investigations of the property, and available information regarding the proposed design and construction concepts for the development. In addition, information from public sources including geologic mapping and MECP well record.
- **Detailed Site Inspection:** An inspection of the property was conducted to review existing Site conditions including identification of any hydrogeological features such as significant areas of potential groundwater recharge or areas of groundwater discharge. The topographic survey of the Site provided to Terraprobe was reviewed in order to provide a discussion regarding drainage conditions.
- **Borehole Drilling:** Prior to the commencement of drilling, the locations of underground utilities; including telephone, natural gas and electrical lines were marked out by local locating companies and individual borehole locations were cleared by private utility locating service providers. The field investigation was conducted from June 13 to 17, 2022 and consisted of drilling and sampling a total of sixteen (16) boreholes extending to depths varying from about 9.2 to 9.4 m below existing ground surface.
- <u>Well Installation</u>: To measure the groundwater level and investigate the quality of groundwater, select eight (8) boreholes (BH1, BH4, BH7, BH9, BH10, BH13, BH15 & BH16) were instrumented with a monitoring well. The monitoring well consisted of a 50 mm diameter PVC screen with a length of PVC riser pipe, 10-ft slotted screen. Upon installation, an elevation survey of the monitoring wells, relative to a local datum, was completed so that relative groundwater flow direction can be assessed. The information obtained from the boreholes was used for the hydrogeological assessment.
- <u>Completion of hydraulic conductivity testing</u>: Single well response tests (Bail Tests) were conducted in the all eight (8) monitoring wells to assess hydraulic conductivity of the screened strata.

- <u>Hydrogeology Report</u>: Following completion of the above-noted study, a detailed engineering report was prepared regarding the Site hydrogeology. The report provides the following information:
  - Presentation of all the factual information gathered during the study including the background information and results of site subsurface investigation;
  - Provision of a conceptual site model for local and regional hydrogeologic conditions. The conceptual site model will be used as a basis to assess impacts to local surface and groundwater features;
  - Finite Element Model Analysis will be conducted to estimate the volume of seepage and ground water flow into the excavation area of the proposed development, as well as permanent sub.
  - o Impact assessment and mitigation measures to maintain the hydrogeological functions;

## 3.0 APPLICABLE REGULATIONS AND POLICIES

## 3.1 Conservation Halton (CH) Policies and Regulations (O. Reg. 179/06)

Under Section 28 of the Conservation Authorities Act, local conservation authorities are mandated to protect the health and integrity of the regional greenspace system and to maintain or improve the hydrological and ecological functions performed by valley and stream corridors. The CH, through its regulatory mandate, is responsible for issuing permits under Ontario Regulation (**Ont. Reg. 179/06**), *Development, Interference with Wetlands and Alterations to Shorelines and Watercourses* for development proposal or Site alteration work to shorelines and watercourses within the regulated areas.

CH Regulated Area online mapping was reviewed as shown in **Appendix A** by Terraprobe. It is our understanding that the proposed development will only include the construction of an 8 Storey building with the potential for 1 level of underground parking and services, new site layout with green space, vehicle circulation and parking surfaces. Based on the extent of development, it is our understanding the property is not located within an CH Regulated Area.

#### 3.2 Clean Water Act 2006

The MECP mandates the protection of existing and future sources of drinking water under the Clean Water Act, 2006 (CWA). Initiatives under the CWA include the delineation of Wellhead Protection Areas (WHPAs), Significant Groundwater Recharge Areas (SGRAs) and Highly Vulnerable Aquifers (HVAs), as well as the assessment of drinking water quality and quantity threats within Source Protection Regions. Source Protection Plans are developed under the CWA and include the restriction and prohibition of certain types of activities and land uses within WHPAs. This plan dictates that any site within the Lake Simcoe Watershed of South Georgian Bay (SGBLS) region can be rated in terms of score indicating vulnerability to drinking water quality and quantity threats. Based on the review of MECP's Source Protection Information Atlas and Conservation Halton (CH) regulated area mapping, the following information was obtained related to the subject property:

Associated Policy Area	Applicability	
Conservation Authority	Conservation Halton	
Source Protection Area	Halton Region Source Protection Area	
Watershed	Sixteen Mile Creek – Credit River Watershed	
Subwatershed	Sixteen Mile Creeks	
LSRCA Regulated Area	Not Regulated as per CH interactive regulation mapping.	
Wellhead Protection Area	No	
(WHPA)		
Significant Groundwater Recharge	No	
Area (SGRA).		
Highly Vulnerable Aquifer (HVA)	No	
Wellhead Protection Areas		
(WHPA - Q) or Recharge	No	
Management Area		

Associated Policy Area	Applicability
Intake Protection Zone	No
Intake Protection Zone Q	No
Oak Ridges Moraine (ORM)	The area of proposed development is not located within the Oak Ridges Moraine (ORM)
Niagara Escarpment Plan Area	No
Greenhelt Protection Act Area	The area of proposed development is not located within the Greenbelt Protection Act
Greenbert Protection / Ret / Nea	Area

Refer to Appendix A for associated regulatory mapping details.

# 3.3 Permit to Take Water (PTTW) Section 34 of the Ontario Water Resource Act

For construction dewatering, water takings of more than 50,000L/day but less than 400,000L/day should be registered on the Environmental Activity and Sector Registry (EASR), while water takings of more than 400,000L/day require a Category 3 PTTW issued by the MECP. If it is identified that an EASR or PTTW is required for the Site, a hydrogeological report will need to be submitted in support of the application.

MECP website was reviewed for any active PTTW application records within a 1.0 km radius of the Site on December 14, 2022. Record review indicates that there are no active PTTW within 1.0 km from the Site. There are no other records of water extraction activities within 1 km of the Site, with the exception of the above noted municipal well activities.

#### 3.4 Hydrogeological Assessment Submission Guidelines 2013

The Conservation Authority Guidelines for Development Applications (June 2013) was reviewed as a part of this assessment. This guidance document provides a list of recommended requirements for hydrogeological investigations. Conservation Halton (CH) has adopted these guidelines for hydrogeological assessments.

# 4.0 DESCRIPTION OF SITE CONDITIONS

## 4.1 Site Location and Description

The Property is located southwest quadrant of the intersection of Regional Road 25 and Louis St Laurent Avenue. The general location of the Site is shown on **Figure 1**. It is understood that the proposed development will include the construction of an 8 Storey building with the potential for 1 level of underground parking and services, new site layout with green space, vehicle circulation and parking surfaces. The proposed development will be municipally serviced for water and sewer.

# 4.2 Site Topography and Drainage

Based on the boreholes advanced the Site elevation varies from approximately  $192.94 \pm \text{masl}$  to  $192.14 \pm \text{masl}$  (meters above sea level) towards south/ southeast. Furthermore, based on the review of Oak Ridges Moraine Groundwater Program (OGRMP), it is also indicated that the Site slopes southeast towards Sixteen Mile Creek trends in a northwest-southeast direction, located approximately 300 m east from the area of proposed addition. The topography of the Site is indicated in **Appendix A**.

The nearest surface water features are Sixteen Mile Creek located approximately 300 m east and a tributary of Sixteen Mile Creek at about 400 m west of the proposed development. Both Creeks flow southeastward following local topography towards Lake Ontario. Regional and local groundwater flow direction is the expected to flow southeast towards Lake Ontario.

# 4.3 Regional Geology and Physiography

The surficial geology of the area is representative of massive to well laminated fine textured 5d: Glaciolacustrine-derived silty to clayey till (5d) with significant amounts of sand. The mapped surficial geology for the Site and the surrounding area is provided in Appendix A.

From a regional perspective, the Site is situated within the physiographic feature known as the Peel Plain (33) and within the physiographic landform known as the 8 Bevelled Till Plains (6). The Peel Plain occupies a central position in the expanded western portion of the South Slope and is separated from the Iroquois shoreline by the Trafalgar Moraine and a strip of fluted till plain. It is generally level-to-undulating with elevations from 150 to 230 masl with a gradual and fairly uniform slope toward Lake Ontario encompassing the majority of the watershed. The plain extends across the contact of the grey and red shales of the Georgian Bay and Queenston Formations, respectively. Consequently, the till is reddish in colour and somewhat lower in lime than the clay in the eastern end of the plain (Chapman, 2007). The underlying Bedrock of the plain is Queenston Formation Shale, limestone, dolostone, siltstone Queenston Formation (55a). The location of the Site within the regional physiography map is provided in Appendix A.

# 4.4 Regional Hydrogeology

The stratigraphy of the surficial deposits within the Sixteen Mile Creek subwatershed is complex as a result of the glacial history. Based on the review of Oak Ridge Moraine Groundwater Program (ORMGP) cross-section tool: the subsurface hydrostratigraphy within the Sixteen Mile Creeks watershed comprises of the following units.

Geological Unit	Brief Description
Recent Glaciolacustrine Deposits	Recent glaciolacustrine deposits are present at the ground surface, which comprise of sandy silt to silt matrix, moderate to high matrix carbonate content, clast content moderate to high Pleistocene deposits. These surficial deposits generally have low to moderate permeabilities and function as an aquitard.
Halton/Kettleby Till	Underlying the recent glaciolacustrine deposits is the Halton/ Kettleby Till which comprises of silt to clayey silt till. The Halton/ Kettleby Till has low permeabilities and functions as an aquitard.
Oak Ridges Aquifer Complex (ORAC) - Regional Aquifer	Underlying the Halton/Kettleby Till is the ORAC, the shallowest aquifer in the region. It predominantly comprises of granular sediments with interlaid finer materials. The ORAC is generally comprised of soils with medium to high permeabilities.
Newmarket Till	Underlying the ORAC is the Newmarket Till, which acts as an aquitard. The Newmarket till comprises of dense sandy silt to clayey silt and generally has a low permeability.

Bedrock was not contacted over the current subsurface investigation. Bedrock in the area is the Queenston shale. Based on the Sixteen Mile Creek Study (2013), The upper 5 m of the shale can be weathered and fractured. The bedrock cap on top of the Milton Outlier consists of the fractured and relatively permeable dolostone of the Guelph-Amabel Formation. The existence of the Queenston shale at or near the surface east of the escarpment has given rise to historical and potential extraction operations.

The thickness of the overburden below the escarpment ranges from 3 to 25 m. The overburden can contain lenses of more permeable sand and gravel. Channelized deposits of sand gravel occur in the lower portion of the subwatershed. These deposits may range from 1.5 to 6 m thick and may be continuous. . Based on the review of Oak Ridges Moraine Groundwater Program (OGRMP) cross section tool, bedrock lies at an elevation of approximately 173.5 masl, and depth to bedrock at the Site is approximately 15 m below ground surface.

# 4.5 Watershed Setting

The Sixteen Mile Creek watershed is one of 18 watershed areas of Conservation Halton, with one for each the creeks and streams that enter Lake Ontario. The Sixteen Mile Creek watershed is about 360 square kilometres of land that includes parts of Milton, Halton Hills, Oakville and Mississauga, and drains into Lake Ontario. (CH, 2022).

#### 4.6 Local Groundwater Resources

MECP Water Well Records (WWRs) were reviewed for the registered wells located at the Site and within 500 m radius of the Site boundaries (study area). Information contained in these records provides data for determining the nature and use of local groundwater resources. A total of 41 well records were found. The locations of the well records in the Study Area are presented on **Figure 3**, with the details for each well summarized in **Appendix B**. A summary of data obtained from these MECP records is presented in Table below:

Total Number of Wells	41
Depth Ranges	
50 ft. or less	12 (29%)
51 ft. to 100 ft.	2 (5%)
Greater than 100 ft.	0 (0%)
Unknown	27 (66%)
Water Use	
Domestic Water Supply	10 (25%)
Public/ Municipal Water Supply	0 (0%)
Commercial Water Supply	1 (2%)
Monitoring/Test Holes	8 (20%)
Not Used/ Abandoned	19 (46%)
Unknown	3 (7%)

The above summary indicates that approximately 25 % of the well records indicate the wells being used for domestic purposes. It is noted that the Site is situated within residential area that is likely serviced by municipal water supply. However, it is expected that there might still be an active use of groundwater for domestic purposes in the vicinity of the Site.

Based on the review of the well records, it is indicated that ten (10) active water supply well were found within 500 m radius of the area of proposed development. The locations of the well records in the Study Area are presented on **Figure 3**. The water supply well record is summarized in the table below, and respective well log is provided in the **Appendix B**.

Map ID	MECP Well ID	Well Depth (m)	Static Water Level (m)	Well Use	Construction Date	Approx. Distance from the Site (m)
4	2804131	10.4	4.6	Domestic	1972-05-26	335
5	2808260	25.6	7.9	Domestic	1993-04-27	295
9	2802505	20.1	7.6	Domestic	1963-05-24	300
12	2802481	10.7	7.6	Domestic	1962-10-28	100
26	2802504	13.7	3.7	Commercial	1956-04-25	155
27	2806272	7.9	0.3	Domestic	1984-05-18	310



31	2809874	10.7	6.0	Domestic	2003-12-10	350
35	2806271	7.9	0.3	Domestic	1984-04-19	390
37	2807518	16.8	9.1	Domestic	1989-11-18	375
38	2802792	11.0	1.2	Domestic	1968-11-09	400
39	2803361	14.0	3.1	Domestic	1970-04-14	400

The area is situated in an urban portion of Milton. The Town of Milton is serviced with piped municipal water. There will be future use of the groundwater resources in the area for water supply purposes. However, based on the record review and nature of the proposed development, it is unlikely that impacts from the proposed development will affect local water resources within the vicinity of the Site.

## 4.7 Property Inspection to Assess Hydrogeologic Features

The Site was assessed for the presence of features which are significant from a hydrogeologic viewpoint to understand the hydrogeologic dynamics of the subject area. In particular, the property was inspected to assess the following:

- Areas of visible groundwater discharge, springs or seepage at the property or in the vicinity of the on-property water courses.
- Areas of potential enhanced groundwater recharge such as closed drainage features or depressions or large flat areas which may allow for significant groundwater infiltration.
- Inspection of swales and drainage courses for evidence of groundwater seepage or springs.
- Evidence of phreatophytic vegetation, which may indicate seasonally high groundwater levels and/or groundwater discharge and seepage.

No significant areas of groundwater recharge (such as depression or kettles) were identified on the subject property.

#### 4.8 Local Surface Water and Natural Heritage Features

Mapping from the Ontario Ministry of Natural Resources and Forestry (MNRF) was to determine if water bodies, wetland and woodland features were present on the Property and within the Study Area. The Ontario Ministry of Natural Resources National Heritage Information Centre database for listings of Areas of Natural or Scientific Interest (ANSIs) was reviewed. The natural heritage map is presented in **Figure 6**. The information is summarized below.



	Property	• No waterbodies are present on the Property		
Water Bodies	Study Area	• The nearest surface water features are Sixteen Mile Creek and one of its tributaries located approximately 300 m east and at about 400 m west of the proposed development. Both Sixteen Mile Creek and its tributary flow southeastward following local topography towards Lake Ontario.		
	Property	<ul> <li>Provincially Significant Evaluated Wetland</li> <li>No Provincially Significant wetlands are present on the Property</li> <li><u>Non- Provincially Significant Evaluated Wetland</u></li> </ul>		
Wetlands		<ul> <li>No Non- Provincially Significant wetlands are present on the Property.</li> <li><u>Unevaluated Wetland</u></li> <li>No wetland features are present within the area of proposed development.</li> </ul>		
	Study Area	<ul> <li>Provincially Significant Evaluated Wetland</li> <li>The closest Provincially Significant Evaluated Wetland is present approximately 1.2 km northeast from the area of proposed development</li> <li>Non- Provincially Significant Evaluated Wetland</li> <li>The closest Non-Provincially Significant Evaluated Wetland</li> <li>The closest Non-Provincially Significant Evaluated Wetland is present approximatel 2.0 km southwest from the area of proposed development</li> <li>Unevaluated Wetland</li> <li>Unevaluated wetland feature is present within the close proximity of the northeast perimeter of the Property which is located approximately 20 m northeast of the area proposed development</li> </ul>		
Property		• Woodland areas are identified on the Property. However, no woodland areas are present within the area of proposed development.		
Woodlands	Study Area	• The closest woodland area was identified in close proximity of the Property along the north and south perimeter.		
ANSIs	Property	<ul> <li><u>Provincially Significant Life Science ANSI</u></li> <li>No Life Science ANSIs were identified on the Property.</li> <li><u>Provincially Significant Earth Science ANSI</u></li> <li>No Earth Science ANSIs were identified on the Property.</li> </ul>		
	Study Area	<ul> <li><u>Provincially Significant Life Science ANSI</u></li> <li>No Life Science ANSIs were identified in the Study Area.</li> <li><u>Provincially Significant Earth Science ANSI</u></li> <li>No Provincially Significant Earth Science ANSIs were identified in the Study Area</li> </ul>		



# 5.0 RESULTS OF SUBSURFACE INVESTIGATION

The field investigation was conducted from June 13 to 17, 2022 and consisted of drilling and sampling a total of sixteen (16) boreholes extending to depths varying from about 9.2 to 9.4 m below existing ground surface.

To measure the groundwater level and investigate the quality of groundwater, eight (8) boreholes (BH1, BH4, BH7, BH9, BH10, BH13, BH15 & BH16) were instrumented with a monitoring well. The locations of boreholes and monitoring wells are shown on the attached **Figure 2**.

The boreholes were drilled by a specialist drilling contractor using truck/track-mounted drilling rigs equipped with power augers. The borings were advanced using continuous flights of solid stem augers and were sampled at 0.75 m interval with conventional 50 mm diameter split barrel samplers. The drilling was conducted under the full-time supervision of a member of our field staff, who logged the borings and examined the samples as they were obtained. All samples obtained during the investigation were sealed into plastic jars, and transported to our geotechnical testing laboratory for detailed inspection and testing.

# 5.1 Local Site Setting

Based on the review of the geotechnical report **File No. 1-22-0209-01**; the subsurface soil stratigraphy is indicated below. The following stratigraphy is based on the borehole findings, as well as the geotechnical laboratory testing conducted on selected representative soil samples. The stratigraphic boundaries indicated on the Borehole Logs are inferred from non-continuous samples and observations of drilling resistance and typically represent a transition from one soil type to another. These boundaries should not be interpreted to represent exact planes of geological change. The subsurface conditions have been confirmed in a series of widely spaced boreholes and will vary between and beyond the borehole locations.

# 5.1.1 Surficial Topsoil/ Earth Fill Material

A surficial layer of **topsoil** was encountered at all borehole locations, and its thickness ranged from 90 mm to 150 mm. The topsoil was underlain by weathered/disturbed native soil in boreholes BH1, BH2, BH3, BH4, BH7, BH12, BH14 and BH16. A zone of earth fill materials was encountered in boreholes BH5, BH6, BH8, BH9, BH10, BH11, BH13 and BH15 and extended to a depth of 0.8 m below grade. The earth fill materials consisted of

clayey silt, trace gravel, trace sand, trace organics. The fill material indicated a soft to stiff relative density with moist condition.



# 5.1.2 Native Soils

Undisturbed native soil deposits underlie the topsoil / earth fill deposits, which extended to the full depth of investigation are as follows:

- **Clayey Silt Till**, with varying amount of sand (some sand to sandy) and trace amounts of gravel was encountered below the fill material in each borehole. Clayey silt till extended to depths of about 3.0 to 6.1 m below grade indicating a moist condition.
- Sandy Silt to Sand and Silt Till, with varying amount of clay and gravel (trace to some) was encountered below the clayey silt till layer in each borehole. The cohesionless till extended to the full depth of investigation (about 9.2 to 9.4 m depth below grade), indicating a moist to wet condition.

The detailed stratigraphic conditions are presented on the accompanying borehole logs provided in **Appendix C**. A subsurface profile of Site is provided in **Figure 4**. Geotechnical characterization of the various soil types, including grain size analysis, was conducted and is presented in **Appendix D**. Additional information pertaining to soil stratigraphy is discussed in the geotechnical report by Terraprobe under a separate cover (**File No. 3-22-0209-01**).

## 5.2 Monitoring Well Installation

Monitoring wells were installed in eight (8) boreholes (BH1, BH4, BH7, BH9, BH10, BH13, BH15 and BH16) for groundwater monitoring and to investigate groundwater quality. The monitoring wells were constructed using 50-mm diameter PVC riser pipes and screens, which were installed in each of the selected geotechnical boreholes in accordance with Ontario Regulation (O. Reg.) 903. Filter sand was placed around the well screen to approximately 0.6 m above the top of the screen. The wells were then backfilled with bentonite to approximately 0.3 m below ground surface. All monitoring wells were surveyed using an R10 Trimble GPS relative to a geodetic datum. The details are provided below:

	Well	Ground Surface	Top of Screen		Bottom of Screen		
Well ID	Diameter (mm)	Elevation (masl)	Depth (mbgs)	Elev. (masl)	Depth (mbgs)	Elev. (masl)	Screened Geological Units
BH 1	50	192.04	6.1	185.94	9.1	182.9	Sandy Silt to Sand & Silt Till
BH 4	50	192.6	6.1	186.49	9.1	183.44	Sandy Silt to Sand & Silt Till
BH 7	50	192.7	6.1	186.6	9.1	183.55	Sandy Silt to Sand & Silt Till
BH 9	50	192.5	6.1	186.4	9.1	183.35	Sandy Silt to Sand & Silt Till
BH 10	50	192.9	6.1	186.77	9.1	183.72	Sandy Silt to Sand & Silt Till



Well ID	Well Diameter	Ground Surface	Top of Screen		Bottom of Screen		Screened Geological Units	
BH 13	50	192.4	6.1	186.28	9.1	183.23	Sandy Silt to Sand & Silt Till	
BH 15	50	192.2	6.1	186.13	9.1	183.08	Sandy Silt to Sand & Silt Till	
BH116	50	192.2	6.1	186.15	9.1	183.1	Sandy Silt to Sand & Silt Till	

Note: masl: meters above sea level, mbgs: meters below ground surface

Additional details of the monitoring well installation is presented on the enclosed borehole logs provided in **Appendix C**.

## 5.3 Groundwater Level Monitoring

Water levels was measured in the installed monitoring wells on the site on one event at the time of hydrogeological testing at the Site. The depth to groundwater measured on July 14, 2022 is provided below.

	Ground	July 14, 2022			
Monitoring Well	Surface Elevation (masl)	Groundwater Depth (mbgs)	Groundwater Elevation (masl)		
BH 1	192.0	6.71	185.33		
BH 4	192.6	6.95	185.64		
BH 7	192.7	7.36	185.34		
BH 9	192.5	6.88	185.62		
BH 10	192.9	7.13	185.73		
BH 13	192.4	6.77	185.60		
BH 15	192.2	7.14	185.09		
BH16	192.2	7.33	184.92		

Note: masl: meters above sea level, mbgs: meters below ground surface

Based on the water level recordings, it is noted that groundwater level in the overburden varies from  $184.9 \pm \text{masl}$  to  $185.7 \pm \text{masl}$  (6.7 to 7.4 mbgs).

It is noted that regional and local groundwater flow direction is expected to mimics the surface topography appears to be in the southeast towards Lake Ontario located approximately 16 km southeast from the area of proposed development. The groundwater flow direction is shown in **Figure 5**.

It should be noted that the groundwater levels noted above may fluctuate seasonally depending on the amount of precipitation and surface runoff. Further, long term groundwater monitoring will be required to capture the seasonal groundwater flow fluctuations.

The monitoring wells installed at the Site need to be maintained in accordance with Ontario Water Resources Act, O. Reg. 903/90. When the wells are no longer required for monitoring or sampling purposes, these wells will need to be appropriately decommissioned by a licensed well contractor as outlined in the Regulation.

# 5.4 Estimation of Hydraulic Conductivity

## 5.4.1 Estimation from Grain Size

In order to estimate the hydraulic conductivity (K) from the grain size distribution curves an excel based tool/program HydrogeoSieveXL (Devlin, J.F. 2015) is used that calculates the hydraulic conductivity from grain size distribution curves using 15 different methods. HydrogeoSieveXL was found to calculate K values essentially identical to those reported in the literature, using the published grain-size distribution curves. This program is developed by J.F Devlin, Department of Geology, University of Kansas (Developed April 29, 2014, most recent update September, 2016). HydrogeoSieveXL presents the completed data table, a grain size distribution curve, an extensive list of grain size characteristics from which effective grain diameters are calculated, a histogram of grain size distribution presented in terms of conventional grain size classes and 15 estimates of K calculated from the formulas. Geometric and arithmetic means of the estimated K values are also calculated. The complete report for each sample is provided in along with the grain size results in **Appendix E**. The results of the estimates are summarized below:

Borehole No./Sample ID	Sampling Depth (mbgs)	Sampling Elevation (masl)	Soil Description (Native)	Estimated Hydraulic Conductivity (m/s) (Geometric Mean)	
Borehole 1, Sample 8	7.7	184.4	Sandy Silt to Sand & Silt Till	9.9 x 10 <sup>-9</sup>	
Borehole 4, Sample 6	4.8	187.8	Clayey Silt Till	1.5 x 10 <sup>-9</sup>	
Borehole 10, Sample 5     3.4     189.5     Clayey Silt Till		Clayey Silt Till	2.2 x 10 <sup>-9</sup>		
Borehole 13, Sample 7	6.2	186.1	Sandy Silt to Sand & Silt Till	1.0 x 10 <sup>-8</sup>	
Borehole 16, Sample 9	9.2	183.1	Sandy Silt to Sand & Silt Till	3.1 x 10 <sup>-7</sup>	

Note: masl: meters above sea level, mbgs: meters below ground surface

Based on the grain size distribution analysis, the hydraulic conductivity of the upper cohesive clayey silt till deposit is estimated in order of  $10^{-9}$  m/s. Moreover, the hydraulic conductivity of the lower sandy silt glacial till is estimated in order ranging from  $10^{-7}$  m/s to  $10^{-8}$  m/s.

# 5.4.2 Estimation from In-situ Hydraulic Testing

The hydraulic conductivity was also determined based on single well response tests (Bail Tests) performed on all monitoring wells (BH1, BH4, BH7, BH9, BH10, BH13, BH15 and BH16). The monitoring wells were developed in advance. Well development involves the purging and removal of

groundwater from the monitoring wells to remove remnants of clay, silt and other debris introduced into the monitoring well during construction, and to induce the flow of formation groundwater through the well screens, thereby improving the transmissivity of the subsoil strata formation at the well screen depths.

The static water level was measured prior to the test. The Solinst Datalogger was programmed to record the water levels at one (1) second of the interval throughout each test. The data from the tests were analysed using Bouwer and Rice method (1967) included in the Aquifer Test V.7 software Package. The results of the analysis are presented in **Appendix E**. The hydraulic properties of the strata applicable to the Site are as follows:

Monitoring Well ID	Top of Well Screen Elevation (masl)	Bottom of Well Screen Elevation (masl)	Screened Geological Units	Hydraulic Conductivity (m/s)
BH 1	185.94	182.9	Sandy Silt to Sand & Silt Till	3.1 x 10 <sup>-8</sup>
BH 4	186.49	183.44	Sandy Silt to Sand & Silt Till	1.84 x 10 <sup>-6</sup>
BH 7	186.6	183.55	Sandy Silt to Sand & Silt Till	3.78 x 10 <sup>-7</sup>
BH 9	186.4	183.35	Sandy Silt to Sand & Silt Till	3.51 x 10 <sup>-8</sup>
BH 10	186.77	183.72	Sandy Silt to Sand & Silt Till	2.99 x 10 <sup>-8</sup>
BH 13	186.28	183.23	Sandy Silt to Sand & Silt Till	1.18 x 10 <sup>-8</sup>
BH 15	186.13	183.08	Sandy Silt to Sand & Silt Till	1.67 x 10 <sup>-7</sup>
BH16	186.15	183.1	Sandy Silt to Sand & Silt Till	3.49 x 10 <sup>-7</sup>

Note: masl: meters above sea level, mbgs: meters below ground surface

Based on the in-situ hydraulic testing, the hydraulic conductivity of the native soils is estimated in order ranging majorly from of  $10^{-6}$  m/s to  $10^{-8}$  m/s, indicating moderate permeabilities

# 5.4.3 Estimation from Literature

According to Freeze and Cherry (1979), the typical hydraulic conductivity of the strata investigated at the site are:

Soil Unit	Estimated Hydraulic Conductivity Range (m/s)		
Earth Fill	10-6		
Glacial Till (Native)	10-6 - 10-12		

Based on the analyses, the hydraulic conductivity calculated from the single well response testing and grain size analyses are consistent with the published values associated with the geological material tested.



## 5.5 Assessment of Groundwater Quality

One (1) unfiltered groundwater sample was collected by Terraprobe and analyzed by a Canadian laboratory accredited and licensed by Standards Council of Canada and or Canadian Association for Laboratory Accreditation. The sample was collected directly from monitoring well BH 16 on July 15, 2022. The monitoring well BH 16 was developed and purged prior to sample collection.

Upon sampling, all of the bottles were placed in ice and packed in a cooler for shipment to the analytical laboratory. Sample analysis was performed by AGAT Laboratories, a laboratory accredited by the Canadian Association for Laboratory Accreditation Inc. (CALA).

The sample was compared to the following:

• Halton Sanitary and Combined Sewer Standards

In summary, the results indicate the following:

• The groundwater sample **meets** the permissible limits for Halton Sanitary & Combined Sewer Guideline values are for all the parameters analyzed.

The results of the groundwater analysis indicate the unfiltered groundwater sample analyzed meet the limits for Region of Halton Sanitary and Combined Sewer Discharge.

The groundwater quality test results and the certificate of analysis are presented in Appendix F.

#### 6.0 IMPACT ASSESSMENT

The impact assessment details that are applicable to the Property are discussed below:

#### 6.1 Groundwater Control and Dewatering Requirements

#### 6.1.1 Construction Requirements and Methodology

The following drawing set was provided to Terraprobe and reviewed in preparation of this engineering report:

• *"Site Plan – Proposed Borehole Locations, Careswell Milton, 6360 Regional Rd 25"*, by Salterpilon, dated February 04, 2022, File No.: 21040.

The proposed development consists of comprised of an 8 storey Long Term Care Home structure with the potential of one level of underground parking and services that would cover most of the site, parking areas and driveways/access routes.

Based on the geotechnical investigation report by Terraprobe File No. 1-22-0209-01 (September 2022), the proposed finished floor elevation (FFE) would be set at about at about 4.0 m depth below grade (Elev.188.5  $\pm$  m). The underside of footing level would likely be set at about 1  $\pm$  m (Elev.187.5  $\pm$  m) below P1 slab.

Based on the review of the geotechnical investigation report prepared by Terraprobe File No. 1-22-0209-01, proposed buildings will be supported on conventional spread footing foundations on the native, undisturbed very stiff to hard clayey silt till deposit. Based on the above stabilized groundwater level recordings on July 14, 2022, it is noted that groundwater level varies from  $184.9 \pm m$  to  $185.73 \pm m$ across the property. The highest groundwater level that will be used in the FEM 186.73 masl applying a 1m fluctuation above the groundwater level recorded during the month of July 2022. This indicates that the the foundation excavations will extend above the prevailing groundwater table at the Site. According to the Geotechnical report, for excavations extending near or below the prevailing groundwater level (e.g. elevator pit), it may be necessary to lower the groundwater level and maintain it at least 1 m below the excavation base prior to and during the subsurface construction. Excavations for foundations will extend a nominal 1.0 m below FFE, and the design dewatering target will be set 1.2 m below the lowest excavation level. As such, it will be necessary to positively depressurize the wet native soil prior to construction for any excavations extending below Elev.  $186.3 \pm m$ .

The permanent drainage system is intended to collect passive groundwater seepage flow from the surrounding soils. For the purpose of dewatering (long term) assessment, the sub drainage system is assumed to be at about 1 foot (0.3 m) below the slab of lowest underground parking, approximately  $Elev.188.2 \pm m$ , which is 1.5 m above the design groundwater level.

**Note:** For further details on the geotechnical design considerations and constructability please refer to geotechnical report prepared by Terraprobe under a separate cover (**File No. 1-22-0209-01**)

## 6.1.2 Groundwater Extraction and Discharge Requirements

A finite element numerical analysis utilizing computer software (Slide 7.014, developed by Rocscience Inc.) was conducted to assess the water taking requirements (short term and long term) in support of the proposed development. The hydraulic parametrization of the generalized numerical model is based on the grain size analyses, field hydraulic testing and published data that incorporates the permeability of different strata encountered during the investigation. The final shoring system and construction method were unknown at this stage. As such, a permeable shoring was assessed for preliminary estimate. The finite element model results are presented in **Appendix G**. For the purpose of accessing groundwater seepage rates, following considerations were made:

- Underside of the foundations set to 1.0 m below the lowest FFE;
- Design water level of Elev. 186.73 ± m, 1m above the measured groundwater level to account for the highest observed water table elevation;

- Design dewatering target set 1.2 m below the foundation's underside;
- Safety factor S.F of 2 is applied on the groundwater flow with the inclusion of 27 mm design rainfall event based on of Intensity-duration-frequency (IDF) curves (Ontario Ministry of Transportation) for 2-hour period for a 2-year event;
- Steady State simulation was performed for dewatering assessment; and
- Hydraulic conductivity for earth fill, native clayey silt glacial till, and sandy silt till assigned as 1 x 10<sup>-6</sup> m/s, 2.2 x 10<sup>-9</sup> m/s, and 1.84 x 10<sup>-6</sup> m/s respectively.

The short-term control of ground water should take into account stormwater management from rainfall events. A dewatering system should take into account the removal of rainfall from the excavation. A value of 27 mm rainfall over a 2-hour period for a 2-year event was used to determine the incidental precipitation volumes to the proposed excavations based on the review of Intensity-duration-frequency (IDF) curves (Ontario Ministry of Transportation). The water taking requirements at the Property are as follows:

Conventional Drained Foundations - Construction (Short Term) – F.S. 2.0						
Scenario	Groundwater Seepage S.F. 1.5 Used		27 mm Design	Rainfall Event	Total Volume (1)	
	L/day	L/min	L/day	L/min	L/day	L/min
Short Term						
(Permeable	28,500	19.8	48,500	33.7	77,000	53.5
Shoring)						
Short Term						
(Impermeable	6,500	4.5	48,500	33.7	55,000	38.2
Shoring)						
Conventional Drained Foundations - Permanent (Long Term) – F.S. 1.5						
Long Term						
(Permeable	0	0	3,000	2.1	3,000	2.1
Shoring)						
Long Term						
(Impermeable	0	0	3,000	2.1	3,000	2.1
Shoring)						

Notes:

(1) – Total volume is rounded up to the nearest thousand

As required by Ontario Regulation 63/16, a plan for discharge must consider the conveyance of stormwater from a 100-year storm. The additional volume based on the excavation area of either Block A/B, C/D or E/F that will be generated in the occurrence of a 100-year storm event (100 mm) is approximately 183,500 L/day (127.4 L/min).

The finite element model results are presented in **Appendix E**.

# 6.1.3 Permitting Requirements

The Ministry of Environment, Conservation and Parks (MECP) regulates construction water taking over 50,000 L/day and less than 400,000 L/day to obtain an Environmental Activity Sector Registry (EASR) Posting the MECP prior to any construction dewatering activities. EASR Registration is required if the taking of water is more than 50,000 liters of ground water, storm water or a combination of ground water and storm water on at least one day during the life of the construction project. Based on the maximum estimated dewatering volumes of 77,000 L/day, an Environmental Activity Sector Registry (EASR) Posting from the MECP prior to the dewatering activities for the proposed works will be required. a short-term permit to take water PTTW will not be required from the MECP prior to the dewatering activities for the proposed works., completed using permeable and impermeable shoring. However, a Sewer Discharge Agreement must be obtained to permit discharge into the City's sewers prior to any temporary construction dewatering discharge.

Furthermore, as the estimated dewatering volumes in long term is larger than 50,000 L/day, a long-term permit to take water PTTW will not be required. However, a Sewer Discharge Permit must be obtained to permit discharge into the City's sewers. It will be necessary to confirm the location of the discharge points, and the capacity of the receiving sewer as part of the permit application. The permit will provide conditions for monitoring of flow and analysis of ground water quality. However, if the proposed development is designed water tight structure, then a private water drainage system will not be required. In such a scenario, the structure must then be designed to resist hydrostatic pressure and uplift forces.

Groundwater taking estimates are based on the assumed excavation dimensions and methodology, therefore should there be changes in these items, there will be a requirement to revise the groundwater taking volumes. It is contractor's responsibility to determine the type and extent of the dewatering system required. It should be noted that the actual required dewatering effort will depend on several factors, including excavation depth, sequencing, season and weather conditions and the length of time the excavation is left open. It should be noted that the interpretation of the dewatering estimates presented in this report and selection of an appropriate dewatering program is the responsibility of contractor.

# 6.1.4 Zone of Influence (ZOI)

The Zone of Influence (ZOI) for dewatering, also known as Radius of Influence ( $\mathbf{R}_0$ ) with respect to maximum drawdown required was calculated based on the estimated groundwater taking rate and the hydraulic conductivity of the unit from which the groundwater will be taken at the Property. The ZOI was calculated using the Sichart's equation below:

Equation:  $R_0 = 3000 * dH * \sqrt{K}$ 

Where:

dH is the dewatering thickness (m) = Highest Observed Water level (m) - Dewatering Target (m)

K is the hydraulic conductivity (m/s) of water bearing deposit

The ZOI for the proposed building excavation with respect to groundwater at the Property is estimated as  $\pm 1.75$  m for short term dewatering.

The above ZOI is estimated by considering the hydraulic conductivity of  $1.84 \times 10^{-6}$  m/sec for the native sandy silt to silt and sand glacial till deposit.

If an impermeable shoring system is considered for the proposed excavation and construction, the ZOI will be limited to the excavation box.

#### 6.2 Surface Water Impact

The nearest surface water features are Sixteen Mile Creek located approximately 300 m east and a tributary of Sixteen Mile Creek at about 400 m west of the proposed development. Both Creeks flow southeastward following local topography towards Lake Ontario. Regional and local groundwater flow direction is the expected to flow southeast towards Lake Ontario. Any groundwater that will be taken from the site will be discharged (if required) into the City's sewer systems and not into any natural water body. As such, there will be no impact to surface water features as a result of proposed development.

#### 6.3 Groundwater Quality Impact

The area of the proposed development is not located in the Highly Vulnerable Aquifer (HVA). HVAs are those areas where an aquifer may be more prone to contamination. The infiltration of rooftop runoff and vegetated surfaces may be permitted if current site conditions demonstrate no existing contamination. Depending on the land use, runoff from urban developments may contain a variety of dilute contaminants such as suspended solids, chloride from road salt, oil and grease, metals, pesticide residues, bacteria and viruses. For groundwater, generally with the exception of the dissolved constituents such as nitrogen and salt, most contaminants are attenuated by filtration during groundwater flow through the soils. Under proposed development, the quality of water directed to pervious areas for infiltration is not expected to contain any contaminant of concerns.

Considering the nature of proposed development, it is anticipated that there will not be any contributing contaminants that may affect the background ground water quality in the area. Phase One and Phase Two Environmental Assessments (ESAs) were completed at the Property. Based on the findings of these studies, no contaminants exceeding the applicable site condition standards were identified at the Property.

## 6.4 Anthropogenic Transport Pathways

No significant anthropogenic transport pathways were identified during investigations conducted on the Property. Eight (8) monitoring wells were installed during site investigation conducted by Terraprobe to gather information regarding the groundwater quality and elevation at the Property. These are all installed in the shallow groundwater, and are therefore not considered to be a risk to the groundwater resource; however, the

wells will be abandoned when no longer in use for monitoring. All monitoring wells will be abandoned prior to the earth works of the proposed development at the Property.

The existence of groundwater transmission pathways is based on the interpretation made solely from the soil type encountered during the subsurface investigation. If any ground water transmission pathways are present on the property, all precautions must be taken to ensure that there is no disruption to the groundwater flow and hydrogeologic functions.

#### 6.5 Dewatering Impacts to Natural Environment

There are no provincially significant, evaluated, surface water features and/or unevaluated wetlands within the close proximity of the Site or within the estimated zone of influence. Furthermore, if the excavation works will be advanced using an impermeable shoring system (caisson wall), the zone of influence will be limited to the excavation box. Any groundwater that will be taken from the site will be discharged (if required) into the Town's sewer systems and not into any natural water body. As such, there will be no impact to the natural environment caused by the water takings at the Property

## 6.6 Local Wells and Zone of Influence

The Site is located in a developed area of the Town of Milton. A review of MECP well records (Section 4.5) indicates that there is a total of 11 water supply wells located within the study area. The water supply wells in the area are predominantly used for domestic water supply (10 records), however there are wells reportedly also used for commercial (1 records) water supply. The closest domestic water supply well (Well ID: 2802481) is located approximately 100 m northeast of the Site. As discussed in Section 7.0, the ZOI from the temporary groundwater control and dewatering activities is expected to be limited in its lateral extent to  $1.75 \pm m$  from the edge of excavation. During the period of groundwater control and dewatering, impacts to nearby users of the groundwater resources in the area is considered unlikely. As such, it is expected that there would be no impact to local groundwater resources in the area.

## 7.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the results of the investigation, the following conclusions and recommendations are provided:

- 1. The property is not located within an CH Regulated Area;
- The property is not located within Wellhead Protection Area and Highly Vulnerable Aquifer (HVA);
- 3. The area of proposed development is not located within Greenbelt Protection Act Area and Oak Ridges Moraine (ORM);

- 4. In summary, the boreholes advanced encountered a surficial topsoil cover underlain by a zone of weathered/disturbed native soil and/or earth fill materials and extended to the depth of 0.8 m below grade respectively. The earth fill materials consisted of consisting of clayey silt, with trace gravel, trace sand, trace organics. Cohesive clayey silt till deposit, with trace to some amounts of sand and gravel was encountered beneath the weathered/disturbed soil/ earth fill zone and extended to depths varying from 3.0 m to about 6.1 m below grade, with low permeability estimated in order of 10<sup>-8</sup> m/s to 10<sup>-9</sup> m/s, followed by cohesionless sandy silt to sand and silt till, with varying amount of clay and gravel extending to the full depth of investigation (about 9.2 to 9.4 m depth below grade). The permeability cohesionless sandy silt till estimated in order of 10<sup>-6</sup> m/s to 10<sup>-7</sup> m/s, indicating moderate permeabilities
- 5. The hydraulic conductivity of the underlying soil stratum at the Site were estimated by completing in-*situ* single well response testing and using the results of the grain size analysis. Based on the results, the hydraulic conductivities of the underlying soil ranged from  $10^{-6}$  m/sec to  $10^{-9}$  m/sec.
- 6. Based on the available water level recordings till date, it is noted that groundwater level in the overburden varies from 184.9± masl to 185.7 ± masl (6.7 mbgs to 7.4 mbgs). The site visit on July 14, 2022 indicates that shallower groundwater recorded is 185.7 ± masl. It is noted that regional and local groundwater flow direction is expected to mimics the surface topography appears to be in the southeast towards Sixteen Mile Creek tributary located approximately 300 m east from the area of proposed addition. It should be noted that the groundwater levels noted above may fluctuate seasonally depending on the amount of precipitation and surface runoff. Further, long term groundwater monitoring will be required to capture the seasonal groundwater flow fluctuations;
- 7. The results of the analytical testing indicated that the quality of groundwater meets the permissible limits for Region of Halton Sanitary and Combined Sewer Discharge for all the parameters analyzed.
- 8. Based on the results of the finite element analysis, the total temporary and permanent groundwater control and dewatering is estimated as follows:

a. Short Term, Permeable – 77,000 L/day (53.5 L/min) b. Short Term, Impermeable – 55,000 L/day (38.2 L/min) c. Long Term, Permeable – 3,000 L/day (2.1 L/min) d.Long Term, Impermeable – 3,000 L/day (2.1 L/min)



# 8.0 CLOSURE

We trust this report meets with your requirements. Should you have any questions regarding the information presented, please do not hesitate to contact our office.

# Yours truly, Terraprobe Inc.

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**Muna MO Mirghani, P.Eng.** Project Manager

Muhammad I Shahid, P. Geo., QP<sub>ESA</sub> Senior Project Manager



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- Terraprobe Inc. (March 2023), Phase Two Environmental Site Assessment, 6360 Regional Road 25, Town of Milton, Ontario. File No, 1-22-0209-42

## LIMITATIONS

This report was prepared by Terraprobe Inc. for the use of **Thomas Robert Colbeck**, and is intended to provide an assessment of the hydrogeological condition on the property located at **6360 Regional Road 25**, **Milton, ON**. The report was prepared for the purpose of identifying the groundwater conditions at the property and any potential groundwater flow cause by either short term construction dewatering or long-term permanent drainage of proposed buildings or structures. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. Terraprobe accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report, including consequential financial effects on transactions or property values, or requirements for follow-up actions and costs.

The assessment should not be considered a comprehensive audit that eliminates all risks. The information presented in this report is based on information collected during the completion of the subsurface investigation conducted by Terraprobe Inc. It is based on conditions at the property at the time of the site inspection. The subsurface conditions were assessed based on information collected at specific borehole and monitoring well locations. The actual subsurface conditions between the sampling points may vary.

There is no warranty expressed or implied by this report regarding the condition of the property. Professional judgment was exercised in gathering and analyzing information collected by our staff, as well as that submitted by others. The conclusions presented are the product of professional care and competence, and cannot be construed as an absolute guarantee.

In the event that during future work new information regarding the condition of the property is encountered, or the proposed development is changed from that which was provided to Terraprobe with respect to the property, Terraprobe should be notified in order that we may re-evaluate the findings of this assessment and provide amendments, as required.

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	Consulting Geotechnical & Environmental Engineering Construction Materials, Inspection & Testing 11 Indell Lane - Brampton Ontario LGT 3Y3 (905) 796-2650
	Reference: Milton GIS
	Notes:
	Project Title:
A Start	Hydrgeological Assessment
es	Site Location:
	6360 Regional Road 25, Milton, Ontario
A	Figure Title:
	Borehole/Monitoring Well Location Plan
10 1 1 0 0 C	Designed By:         File No.:           AQ         1-22-0209-46
· · ·	Drawn By: AA Scale:
	Reviewed By: MS
	Date: December 2022









**Conservation Halton Regulation Mapping** 



Headwater Floodplain Hazard Spill Lines

Meander Belt Hazard

Consult Conservation Halton

Stable Top of Bank (STOB) Hazard

Wetland Hazard

Spill Zone Hazards

 $\overline{Z}$ 

Approximate Regulation Limit

Shoreline Dynamic Beach Hazard

Shoreline 100 year Flood Elevation Hazard

Esri Community Maps Contributors, Province of Ontario, Esri Canada, Esri, HERE, Garmin, SafeGraph, GeoTechnologies, Inc, METI/NASA, USGS, EPA, NPS, US Census Bureau, USDA, NRCan, Parks Canada, Conservation Halton, 2022, Town of Oakville, Maxar, Microsoft

0.04

0.02

0.08 km


## Source Water Protection Map





Page	1	of	1
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				Ν	ECP Well Records	Summary				
		Well Usa;			Jsage	Water Found	Static Water Level	Top of Screen Depth	Bottom of Screen	
WELL ID	MECP* WWR ID	Construction Method	Well Depth (m)**	Final Status	First Use	(mbgs)**	(mbgs)**	(mbgs)**	Depth (mbgs)**	Date Completed
1	7199036		2.5	Abandoned-Other						
2	7343126									29/05/2019
3	7159099			Abandoned-Other						30/11/2010
4	2804131	Boring		Water Supply	Domestic	6.1	4.6			26/05/1972
5	2808260	Cable Tool		Water Supply	Domestic	22.9	7.9			27/04/1993
6	7243822			Abandoned-Other						10/06/2015
7	7243291			Abandoned-Other						10/06/2015
8	7171558									31/07/2011
9	2802505	Cable Tool		Water Supply	Domestic	19.5	7.6			24/05/1963
10	7153622			Abandoned-Other						02/09/2010
11	7243290			Abandoned-Other			7.0			10/06/2015
12	2802481	Boring		Water Supply	Domestic	9.2	7.6			28/10/1962
13	7253736			Abandoned-Other						29/11/2015
14	7303444	Boring	7.6	Monitoring and Test Hole	Test Hole			4.6	7.6	29/08/2017
15	7303447	Boring	3.8	Monitoring and Test Hole	Test Hole			2.3	3.8	29/08/2017
16	7253737			Abandoned-Other						29/11/2015
17	7303446	Boring	6.1	Monitoring and Test Hole	Test Hole			3.0	6.1	28/08/2017
18	2808294	Cable Tool		Test Hole						18/11/1994
19	2808295	Cable Tool		Test Hole						16/11/1994
20	2808297	Cable Tool		Test Hole						15/12/1994
21	7136532		Aban	doned Monitoring and Tes	Monitoring	5.3				27/11/2009
22	7273737			Abandoned-Other						12/10/2016
23	7303445	Boring	7.6	Monitoring and Test Hole	Test Hole			4.6	7.6	30/08/2017
24	7136533		Aban	doned Monitoring and Tes	Monitoring	5.7				30/11/2009
25	7158986									29/11/2010
26	2802504	Cable Tool		Water Supply	Commerical	13.7	3.7			25/04/1956
27	2806272	Boring		Water Supply	Domestic	4.3	0.3			18/05/1984
28	7243289			Abandoned-Other			6.1			10/06/2015
29	7253735			Abandoned-Other						29/11/2015
30	7253742			Abandoned-Other						29/11/2015
31	2809874	Cable Tool	10.7	Water Supply	Domestic	10.0	6.0			10/12/2003
32	2809813	Boring		Abandoned-Supply	Not Used					08/09/2003
33	7301919			Abandoned-Other						28/11/2017
34	2803410	Cable Tool	22.0	Observation Wells	Not Used	12.2	3.4	21.4	22.0	08/04/1970
35	2806271	Boring		Water Supply	Domestic	4.6	0.3			19/04/1984
36	7273736			Abandoned-Other						12/10/2016
37	2807518	Rotary (Convent.)		Water Supply	Domestic	10.4	9.2			18/11/1989
38	2802792	Boring		Water Supply	Domestic	7.6	1.2			09/11/1968
39	2803361	Boring		Water Supply	Domestic	13.4	3.1			14/04/1970
40	7301920			Abandoned-Other						28/11/2017
41	7301921			Abandoned-Other						28/11/2017

UTM 122 31913101913 E	30 M	5-F	6-	57/
X S R 481/163415 N			a. 28 WN	DE BRANZA T
Elev AR BIGIGIZASE The Ontario Water Reso	ources Commission	Act	41011	
Basin d ZUT   WATER WEI	LL REC	ORD	ONTARIO (	WAVER
County or District Halton	fownship, Village, J	own or City	RESURCES CO	IMILES YON
Con 2 CLEN Lot T & I	Date completed	28	Oct	1962.
Owner	ddress M	(day	month	year)
(print in block letters)	C			
Logical diameter of againg 30		Pumpir	ng Test	
Total length of gooing	Static level	~	د م	
	Test-pumping ra	ate	212/	G.P.M.
Length of some	Pumping level		311	
Dopth to top of some	Duration of test j	oumping	<del>~ 7</del> _ /	
Diamatan of finished hele <b>30</b> <sup>4</sup>	Water clear or cle	oudy at end of	test	
	Recommended p	oumping rate.	マイ	G.P.M.
Well Log	with pump settin	g of	feet belo	ow ground surface
	Timere	m	Depth(s) at	Kind of water
Overburden and Bedrock Record	from ft.	10 ft.	which water(s) found	(fresh, salty, sulphur)
Remain				
- Brown May	0		30'	Inal
Blue clay	5	35'		presso
For what purpose(s) is the water to be used? Dorutie		Location	Well S-LOY	<u> </u>
	In diagram	1 below show	istances of we	ll from
Is well on upland, in valley, or on hillside? upland	road and	lot line. Ind	tate north by $5/4$	arrow.
Drilling or Boring Firm		د.	KISA3A	
			1	
Address flavis forme			۲. L	
Newmarket	E		<u>  </u>	\/
Licence Number	10 20	le RI		
Name of Driller or Borer Applaance			8	
Address Mavie Muimarket	4		2	
Date 2007. 8 162.			N	1
(Signature of ignand Drilling on Paring Contractor)			-	
Form 7, 15M Sets _ 60 5020		1		$\times$ 7
1 0111 ( 1011 Sets 00-0990		<i>~ ¥</i>		-Cil-
OWRC COPY			CSS.58	

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	The Wate De	ONTAG er-well Dri partment	illers Act, 195 of Mines	4	JUN 2 0 GEULUGICA	1953 BRANCH
County or Territorial District	Vater- HA⊦To	We		cord	DEPARTMEN OAX	VILLE ALSAR
			Village, To	wn or City 5 <i>llta</i> n	1)	
(day)	(month)	(year)				
Pipe and Casing	Record			P	umping Test	
Casing diameter(s)			Static level . Pumping rate Pumping leve Duration of t	eع el	fm.	
				W	ater Record	
Overburden and Bedrock Record	From ft.	To ft.	Dept at w wate fou	h(s) hich or(s) ind	No. of feet water rises	Kind of water (fresh, salty, or sulphur)
- clay	0	25				
sond & fine gravel	25 <del>7</del>	45-	4.5		55	- Cleargrich
For what purpose(s) is the water Macrose Look	to be used? & ·		In diagra	Loca am below s	tion of Well how distances o	f well from
Is water clear or cloudy?	hillside ? foregoing are true.		road and . Long S &	l lot line.	Indicate north	by arrow.
Form 5		=		# 5~	ideroad	CSS.58

YTM 1/17 2 3793101615 E 5 R 481/651712 N	<i>301</i>		ROUND WATER	BRAMC'S 8505
Basin 124 A WATER WEL	L REC		ONTARIO WA Resources Cam	TER
County or District Halton T	ownship, Village, T	own or City	Esquesing	
Con. 3 Lot 9 D	ate completed	24 (day	May	1963 year)
	ress 287 Jo	nes St.,	Bronte	
Casing and Screen Record		Pumping	g Test	
Inside diameter of casing 6 in.	Static level		25 IT.	
Total length of casing 62 ft.	Test-pumping ra	.te	22	G.P.M.
Type of screen	Pumping level		25 ft.	
Length of screen	Duration of test p	oumping	1 hour	
Depth to top of screen	Water clear or cle	oudy at end of	test clear	
Diameter of finished hole 6 in.	Recommended p	oumping rate.	21/2	G.P.M.
	with pump settin	ng of <b>64</b>	feet below	w ground surface
Well Log			Water	Record
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	Kind of water (fresh, salty, sulphur)
Grey clay	0	51		
Brown clay and gravel	51	<u>50</u> 61		
Red_clay Red_shale	61	66	64 ft.	fresh
				1
Domostio			of Woll	
For what purpose(s) is the water to be used? Domes tic	Indiagra	m below show	distances of wel	ll from
Is well on upland, in valley, or on hillside? Upland	road and	lot line. Ind	licate north by	arrow.
Drilling or Boring Firm J. B. Ruttan		1/+	energiste and energiste spectra and a second se	le ferne agger en forstag - sous ar early fast of the forst form
		A TEEN		- 5 A
Address R.R.#2,	5-91	710	The second secon	
Milton, Ont.	N N	Aut	e (n	
Licence Number 1013		1.13	<u>/</u> 0	*
Name of Driller or Borer Same	<u>5</u> .	1	/ '	
Address		12	1-6	X
Date May 25/63		Y	16'	
(Signature of Licensed Drilling or Boring Contractor)	Boy	INE	1075'	
Form 7 10M-62-1152			CSS	.88
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Water clear or cloudy at end of test.	CLEA	R
Depth to top of screen Recommended pumping rate	3	G.P.M.
biameter of finished hole	feet below	v ground surface
Well Log	Water	Record
Overburden and Bedrock Record     From ft.     To ft.	epth(s) at ch water(s) found	Kind of water (fresh, salty, sulphur)
$To \rho \leq \rho_{1}$	251	FRESH
GREY AND BLUE CLAY 2 24		
For what purpose(s) is the water to be used?	ances of wel	ll from
DOMESTIC Is well on upland, in valley, or on hillside? UPLAND Drilling or Boring Firm MILTON WELL BORING Address 6751 WALKERS LINE	e north by	arrow.
R. # 2. MILTON   Licence Number   Name of Driller or Borer   M PELTIER   Address   SAME   Address   SAME   Address   SAME   Max   (Signature of dicensed Drilling or Boring Contractor)   Form 7 15M-60-4138	-0 -0 	ILTON VALE FARM LANL

	The Ontario Water Resource	es Commission Act 307	15F.
	ATER WELL		JS
Water management in Ontario 1. PRINT ONLY IN SP	ACES PROVIDED	803307 28 GQST C	
COUNTY OR DESTRICT	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	CON., BLOCK, TRACT, SURVEY, ETC.	irvey 007
HAITON	OHKVILLE		MPLETED 048-53
	R. I MILION	LEVATION RC. BASIN CODE I	
<u> </u>	1/10 $1/10$ $4/25$		47
LO	G OF OVERBURDEN AND BEDROCK	( MATERIALS (SEE INSTRUCTIONS)	DEPTH - FEET
GENERAL COLOUR COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	FROM TO
BLACK TOPSOIL		UEOD	1.4.16
BROWN CLAY	Contraction Providence	HOOD	16 21
GREY CLAY	STONES		21 42
BROWN CLAY	STUNES	HABD	42 46
RED SHALE			
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		3 3 1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	5 75 80 34-38 LENGTH 39-40
41 WATER RECORD	51 CASING & OPEN HOLE		
WATER FOUND AT - FEET KIND OF WATER	INSIDE MATERIAL THICKNESS FROM	A TO A MATERIAL AND TYPE	DEPTH TO TOP 41-54
0044 2 SALTY 4 MINERAL	1 10 STEEL 12 2 GALVANIZED 2		EALING RECORD
15-18 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL		20-23 DEPTH SET AT - FEET MATERIAL	AND TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
20-23 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL	2 GALVANIZED 3 CONCRETE	FROM TO	
25-28 1 FRESH 3 SULPHUR 2 SALTY 4 MINERAL	4 OPEN HOLE	27-30 18-21 22-25	
30-33 1 FRESH 3 SULPHUR 34		26-29 30-33 80	
		LOCATION OF V	VELL
	15-16 17-18 HOURS MINS.	IN DIAGRAM BELOW SHOW DISTANCES OF WE	LL FROM ROAD AND
STATIC WATER LEVEL 25 END OF LEVEL PUMPING		LOT LINE. INDICATE NORTH OF MARKEN	
P 0462224 15 MINU	26-28 0.4 6 0.4 6 35-37	MILTON-4M	
U IF FLOWING, 38-41 PUMP INTA	KE SET AT WATER AT END OF TEST 42		N
RECOMMENDED PUMP TYPE RECOMMEN	TEET CLEAR - CLOODI		$\mathbf{\Lambda}$
SHALLOW X DEEP SETTING			
54 SP			
FINAL 2 OBSERVATION STATUS 3 TEXT HOLE	WELL 6 ABANDONED, POOR QUALITY 7 UNFINISHED	<b>2</b>	
OF WELL 4 RECHARGE WE		7001-1	
	6 MUNICIPAL 7 DELIC SUPPLY	3	MILTONVAL
	8 COOLING OR AIR CONDITIONING 9 NOT USED	1 <sup>1</sup> <del>1</del>	E GIVES
57 1 CABLE TOOL	6 BORING	N 4	
METHOD         2 □ ROTARY (CON           OF         3 □ ROTARY (REVI	VENTIONAL) 7 🗌 DIAMOND ERSE) 8 🗋 JETTING	V A A STATE	
DRILLING 4 C ROTARY (AIR) 5 AIR PERCUSSI		DRILLERS REMARKS: 1	PECEIVED 63-68
A NAME OF WELL CONTRACTOR	LICENCE NUMBER	DATA SOURCE / 58 CONTRACTOR 59-62 DATE	210570
ADDRESS		DATE OF INSPECTION INSPECTOR	16
A 6751 WALKERS	LINE J.M. & MILION	S REMARKS:	· · ·
Z M, PELTIER	SUBMISSION DATE	HCE .	CSS.SR
U SIGNATURE OF CONTRACTOR	DAY 20 NO TPR . YR TO	<b>ö</b>	
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	PACES PROVIDED	12804131	MUNICIP. C	
2. CHECK X CORREC	TOWNSHIP, BOROUGH, CTIT, TOWN, WILLACE		BLOCH, TRACT, SURVEY, ETC.	5 <u>22 23 24</u> LOT 25-27
HALTON	DAKUILLE	- mon Million has	Lett.	009
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			A OF OPENING 31-33	65 75 80 Diameter 34-38 Length 39-40
WATER FOUND AT - FEET KIND OF WATER		DEPTH - FEET	NO. >	INCHES FEET
10-13 FRESH 3 USULPHUR 14	INCHES INCHES	FROM TO 50 MATER	RIAL AND TYPE	DEPTH TO TOP 41-44 80 OF SCREEN
M 2 15-18 1 FRESH 3 USUPHUR 19	30 <sup>2</sup> GALVANIZED 3 CONCRETE 2-9	0 424		
20-23 1 C 50501 3 C 501 24	4 OPEN HOLE	20-23 DEPTH S	ET AT - FEET MATERIA	AND TYPE (CEMENT GROUT,
2 SALTY 4 MINERAL	<sup>2</sup> GALVANIZED <sup>3</sup> CONCRETE	FROM 10-	TO 13 14-17	LEAD PACKER, ETC.)
23-28 1 _ FRESH 3 _ SULPHUR 29 2 _ SALTY 4 _ MINERAL	4 [] OPEN HOLE 24-25 1 [] STEEL 26	27-30 18-	21 22-25	
30-33 1 🗇 FRESH 3 🗍 SULPHUR <sup>34</sup> 80 2 🗍 SALTY 4 🖓 MINERAL	2 GALVANIZED 3 CONCRETE	26-2	29 30-33 80	
UMPING TEST METHOD 10 TOTAL		J [J []		
	GPM	L (	OCATION OF W	ELL
STATIC WATER LEVEL 25 LEVEL END OF WATER LEVE PUMPING	ELS DURING 2 RECOVERY	IN DIAGRAM BELO LOT LINE. INDI	W SHOW DISTANCES OF W	ELL PROM ROAD AND
19-21 22-24 15 MINUTES 22-24 15 MINUTES 7 22-24 7 22-2	30 MINUTES 45 MINUTES 60 MINUTES			
T FLOWING 38-41 PUMP INTAKE SET	AT WATER AT END OF TEST 42		LOT 10	
	33 FEET 1 Kelear 2 CLOUDY		100'	LDT9
RECOMMENDER PUMP TYPE RECOMMENDED	133 33 FEET RATE DOOM 46-49 PUMPING DOOM GPM	N	I-A	
50-53	IC CAPACITY			
FINAL	5 ABANDONED, INSUFFICIENT SUPPLY	11 _	150	
STATUS OF WELL 4 TRECHARGE WELL	• REANDONED, POOR QUALITY 7 [] UNFINISHED			
55-56 1 X DOMESTIC	5 COMMERCIAL		+6 min	ICK.
WATER 2 STOCK 3 3 I IRRIGATION 7	5 🔲 MUNICIPAL 7 📋 PUBLIC SUPPLY			
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S7 1 CABLE TOOL	6 KBORING	<b>↓</b>	#25 Mug	
OF 3 CROTARY (CONVENTION	AL) 7 DIAMOND 8 JETTING			
DRILLING 4 COTARY (AIR) 5 AIR PERCUSSION	9 🗋 DRIVING 🦾 🦟	DRILLERS REMARKS:		
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ADDRESS	OKING 3637		3637	100579
6251 WALKERShi	WE KR2 Mician		morector	ZN
NAME OF DRILLER OR BORER PEUT	IFI 3637			PAR
SIGNATURE OF CONTRACTOR	SUBMISSION DATE	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	CSS.S	8
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	374 Mar	125 MILTON	DAY MO YR
<u> </u>	ING RC.	ELEVATION RC BASIN CODE	
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GENERAL COLOUR MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	FROM TO
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	51 CASING & OPEN HOLE BE		65         75         80           31-33         DIAMETER         34-38         LENGTH         39-40
WATER FOUND AT - FEET KIND OF WATER	INSIDE MATERIAL THICKNESS FROM	PTH - FEET	INCHES FEET DEPTH TO TOP 41-44 30
10-13 1 pg FRESH 3 _ SULPHUR 4 2 SALTY 4 _ MINERAL	INCHES         INCHES         INCHES         INCHES           10-11         1         STEEL         12           2         GALVANIZED         7         7		OF SCREEN
15-18 1 2 FRESH 3 SULPHUR 19	2.0 ISC CONCRETE 4 OPEN HOLE	61 PLUGGING	6 & SEALING RECORD
20-23 1 _ FRESH 3 _ SULPHUR 24 2 _ SALTY 4 _ MINERAL	$\begin{array}{c c} 17 10 & 1 \\ \hline & \\ 3 \\ \hline & \\ \hline & \\ 3 \\ \hline & \\ \hline \\ \hline$	20-21 DEPTH SET AT - FEET M FROM TO	ATERIAL AND TYPE (CEMENT GROUT LEAD PACKER, ETC.)
25-28 1 _ FRESH 3 _ SULPHUR 29 2 _ SALTY 4 _ MINERAL	4 C OPEN HOLE	27-30 18-21 22-25	
30-33 1 [] FRESH 3 [] SULPHUR 34 4C 2 [] SALTY 4 [] MINERAL	2 GALVANIZED     3 GONCRETE     4 OPEN HOLE	26-29 30-33 80	
71 PUMPING TEST METHOD 10 PUMPING RATE	11-14 DURATION OF PUMPING	LOCATION O	FWELL
T PUNP 2 2 BAILER STATIC WATER LEVEL 25 END OF WATER LE	GPM HOURS MINS	IN DIAGRAM BELOW SHOW DISTANCE: LOT LINE INDICATE NORTH BY AR	S OF WELL FROM ROAD AND Row.
LEVEL PUMPING 19-21 22-24 15 MINUTES 26-28	2   RECOVERY 30 MINUTES 45 MINUTES 50 MINUTES G29-31 32-34 - 35-37		
FEET         FEET         FEET           IF FLOWING,         38-41         PUMP INTAKE ST           GIVE RATE         CI         CI	FEET SPEET ZEET		
G GPM GPM RECOMMENDED PUNP TYPE RECOMMENDED	FEET 1 CLEAR 2 CLOUDY	090 110 03100 4	X
C PUMP SHALLOW DEEP SETTING	PUMPING 62 GHM	03000	tol.m.
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STATUS 2 DOBSERVATION WELL STATUS 3 D TEST HOLE OF WELL A D RECHARGE WELL	6 ABANDONED POOR QUALITY 7 UNFINISHED		
SS-S6 1 A DOMESTIC			Roz Ma
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METHOD 2 CABLE TOOL 2 ROTARY (CONVENTI OF 3 ROTARY (REVERSE)	BORING ONALI     7      DIAMOND     JETTING	257	
DRILLING 4 CONTARY (AIR) 5 AIR PERCUSSION	9 🗌 DRIVING	DRILLERS REMARKS	
C MAME OF WELL CONTRACTOR	BORING 3622	DATA SS CONTRACTOR 59-62	COAQE 63-44 10
OL ADDRESS	LIND RAMINI	DATE OF INSPECTION INSPECTOR	
NAME OF DRILLER OR BORER	LICENCE NUMBER		
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Langue y	DAY CI W YR	<u> </u>	FORM NO. 0506-4-77 FORM 7

Ministry		The Ontario Water Resources Act	30 M/5 f
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	374 Mu	425 MILTON DAY	MO YR 84
1 2 <sup>11</sup> 10 12			
		OCK MATERIALS (SEE INSTRUCTIONS)	DEPTH · FEET
GENERAL COLOUR COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	FROM TO
DROWN IDPSD	1 6-	ACKAD	1 14
" GUNGRI	UNE SIDNES GREY	1 Beuldar !!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!	14 15
BLUE CLAY		SAT	15 25
GREY GRAVEL	SAND STONES	hoose	25 26
		•	
$\begin{bmatrix} 32 \\ 1 \\ 2 \end{bmatrix} \begin{bmatrix} 10 \\ 14 \\ 15 \end{bmatrix} \begin{bmatrix} 10 \\ 21 \\ 21 \end{bmatrix}$		BECOBD	34-38 LENGTH 39-40
WATER FOUND AT - FEET KIND OF WATER	INSIDE WALL DIAM MATERIAL THICKNESS	DEPTH - FEET U	INCHES FEET TH TO TOP 41-44 3
10-13 <sup>1</sup> gr FRESH <sup>3</sup> □ SULPHUR <sup>2</sup> □ SALTY <sup>4</sup> □ MINERAL	10-11 T STEEL 12		FEET
15-10 1 DK FRESH 3 D SULPHUR 2 D SALTY 4 MINERAL	<sup>19</sup> 30 <sup>1</sup> D <sup>2</sup> CONCRETE 2 ± 2	6 23 61 PLUGGING & SEALING	G RECORD
20-23 1 _ FRESH 3 _ SULPHUR 2 _ SALTY 4 _ MINERAL	ZA STEEL STEEL STEEL STEEL STEEL	22 2 6 FROM TO MATERIAL AND TYPE	E (CEMENT GROUT
25-28 1 _ FRESH 3 _ SULPHUR 2 _ SALTY 4 _ MINERAL	23 4 OPEN HOLE	27-30 18-21 22-25	
30-33 1 [] FRESH 3 [] SULPHUR 2 [] SALTY 4 [] MINERAL	2 □ GALVANIZED           3 □ CONCRETE           4 □ OPEN HOLE	26-28 30-33 80	
71 PUMPING TEST METHOD 10 PUMPING	RATE 11-14 DURATION OF PUMPING	LOCATION OF WELL	·····
1 D PUMP 2 2 BAILER STATIC WATER LEVEL 25		IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM	M ROAD AND
LEVEL PUMPING 19-21 22-24 15 MIN W	Z         RECOVERY           UTES         30 MINUTES         45 MINUTES         60 MINUTES           26-28         29-31         / 32-34         //35-37	090 100 2000	AN
S FEET G	FEET FEET FEET FEET FEET FEET FEET FEET	13/80	
	FIDED 43-45 RECOMMENDED 45-45		
C PUMP SETTING	22 FEET PUMPING RATE 6 GPM	55	
FINAL STATUS 3 I TEST HOLE	N WELL 6 ABANDONED POOR QUALITY 7 UNFINISHED	0.8	
OF WELL 4 □ RECHARGE W 55-56 1 D DOMESTIC	5 🗌 COMMERCIAL	Pist	
WATER 2 D STOCK 3 D IRRIGATION	MUNICIPAL     DUBLIC SUPPLY     CONTINUE		ROANNISR
	<sup>9</sup> NOT USED		angi - Tao angi aking Ni Yanganin mininggan pengartantar tarapa
METHOD 2 ROTARY (COL	• DE BORING		
DRILLING	ION	DRILLERS REMARKS	
NAME OF WELL CONTRACTOR	LICENCE NUMBER	DATA 58 CONTRACTOR 59-62 DATECRED	165"
ADDRESS	IL DORING SG37		<u>+ QJ  </u>
NAME OF DRILLER OR BORER	15 GINE KREMICI	D REMARKS	
O SIGNATURE OF CONTRACTOR	FUN ER 5637		
Vinne to	DAY A/ MAMMY YR 8		M NO. 0506-4-77 FORM
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									<u> </u>	47 
GENERAL COLOUR	MOST	OTHER MAT	ERIALS			GENE	ERAL DESCRIPTION		DEPTH	FEET TO
BROWN	SANDY CLAY		· · · · · · · · · · · · · · · · · · ·		LOC	OSE			0	30
RED	CLAY	GRAVEL			LO	OSE			30	34
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41 WAT WATER FOUND AT - FEET	KIND OF WATER	INSIDE DIAM INCHES MATERIAL	OPEN HO		ECORD EPTH - FEET		ATERIAL AND TYPE		INCHES DEPTH TO TOP OF SCREEN	FEET 41-44 30
34	] FRESH 3 🗆 SULPHUR ] SALTY 4 🗆 MINERALS 6 🗆 GAS	10-11 LE STEEL 12 C GALVANIZED	188	<b>#</b> 1	13-16 3/L	Š				FEET
15-10 1 C 2 C	] FRESH 3 SULPHUR 19 4 MINERALS 3 SALTY 6 GAS	4 □ OPEN HOLE 5 □ PLASTIC	9		20-23	61 DEPT		G & SEAI		)RD
20-23 1 C 2 C	] FRESH 3 DSULPHUR 24 4 DMINERALS SALTY 6 DGAS	1 USTEEL 2 GALVANIZED 3 CONCRETE 40 OPEN HOLE		3	4 55	FRU	10-13 14-17		LEAD P	ACKER. ETC >
25-28 1 C 2 C 30-33 1 C 2 C	] FRESH 3SULPHUR 29 ] SALTY 4MINERALS GGAS ] FRESH 3SULPHUR 34 10 4MINERALS ] SALTY 6GAS	5 □ PLASTIC 24-25 1 □ STEEL 2 □ GALVANIZED 3 □ CONCRETE 4 □ OPEN HOLE 5 □ PLASTIC	6		27-30		18-21 22-25 26-29 30-33 80			
71 PUMPING TEST MET	THOD 10 PUMPING RATE	11-14 DURATION OF P	UMPING				LOCATION C	FWEL	L	
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U 30 FEET GIVE RATE	55 FEET 55220 38-41 PUMP INTAKE S	T 55-31 55 FEET 55 FEE	-34 EET 52	42				1		
RECOMMENDED PU	GPM MP TYPE RECOMMENDED PUMP V DEEP SETTING	FEET 43-45 PUMPING FEET RATE	4	6-49 GPM				MILE	ĥ	1
FINAL STATUS OF WELL	<ul> <li>WATER SUPPLY</li> <li>OBSERVATION WELL</li> <li>TEST HOLE</li> <li>RECHARGE WELL</li> </ul>	S ABANDONED, INSU C ABANDONED POOF 7 UNFINISHED D DEWATERING	IFFICIENT SUPP R QUALITY	PLY			HINY # #25.	, , , , , , , , , , , , , , , , , , ,		-
WATER USE	5-56 1 (1) DOMESTIC 2 STOCK 3 IRRIGATION 4 INDUSTRIAL 0 OTHER	COMMERCIAL     MUNICIPAL     PUBLIC SUPPLY     COOLING OR AIR COND     COOLING OR AIR COND     P      NO	DITIONING		-		N+	E		NELL
METHOD OF CONSTRUCTIO	S7     1     CABLE TOOL     2     ROTARY (CONVENT     )     ROTARY (REVENSE)     ON     4     ROTARY (AIR)     5     AIR PERCUSSION	IGNAL)	ОТНЕР			ĸc			′ <u> </u>	] 716
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<b>e</b> revi	OUS DRILLE	D WELL								75
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	ESH 3 DSULPHUR LTY 4 DMINERALS 6 DGAS	10-11 1 STEEL 2 GALVAN	12	75	84					FEET
15-10 <sup>1</sup> FRI 2 SAI	ESH 3 $\Box$ SULPHUR <sup>19</sup> 4 $\Box$ MINERALS LTY 6 $\Box$ Gas	O 3 □CONCRE 4 DOPEN H 5 □ PLASTIC	TE OLE			61	PLUGGIN	G & SEAL	ING REC	ORD
20-23 1 FRI	ESH 3 SULPHUR 24	17-18 1  STEEL 2  GALVAN 3  GONGRE	19 12ED		20-2	FROM		MATERIAL AND	TYPE CEN	ENT GROUT PACKER, ETC )
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2 C SAI	4 DMINERALS LTY 6 DGAS	4 DOPEN H 5 DPLASTIC	OLE							
71 PUMPING TEST METHOD	10 PUMPING RATE	11-14 DURATIO	0N OF PUMPING	17-18			LOCATION C	F WELI	L	
STATIC WA	TER LEVEL 25 END OF WATER L	GPM	I DURS	MINS	IN D	IAGRAM BEI LINE IN	LOW SHOW DISTANCI	S OF WELL I	FROM ROAD	AND
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Е 26 FEET	54.ET 35 FE	1 43 FET	54 54	FEET		_	$a \mid$			
C IF FLOWING. GIVE RATE	30-41 PUMP INTAKE	,X	AT END OF TEST		L	07	3.			
	PE RECOMMENDED PUMP	70 43-45 RECOMI	MENDED A	6-49	C	ON .	5			
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EINAI S4	1 X WATER SUPPLY	s 🗌 ABANDONEI	D. INSUFFICIENT SUP	PLY	1ºH		×			ONT.
STATUS	2 OBSERVATION WEL 3 C TEST HOLE	L 6 ABANDONEC , 7 UNFINISHE	D POOR QUALITY		X					
55-56	I RECHARGE WELL      Z DOMESTIC	DEWATERING 5 🗍 COMMERCIAL	G		WELL		У			
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OF METHOD	2 ROTARY (CONVENT 3 ROTARY (REVERSE	() 7 Di	AMOND TTING						7	1979
CONSTRUCTION	4 C ROTARY (AIR) 5 AIR PERCUSSION	9 🗆 DR	IVING GGING OTHER	DF	RILLERS REMA	RKS				
NAME OF WELL CONT	RACTOR		WELL CONTRACT		DATA	58	CONTRACTOR 59-62	DATE RECEIVED		63-68 80
CORE	S WELL DRI	LLING	1660		DATE OF INS	PECTION		AUG	2 4 19	]4
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<b>Ontario</b>	Ministry of the Environment	Well Tag	g Number	ja <b>č</b> it	3866	Regulation 90	We 3 Ontario Wate	II R r Reso	ecord
Instructions for Complete	na Form	A	003	888				bage	of
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Address of Well Location (Coun	y/District/Municipalit	ty)	Te	wnship	110.1	Lot	- Conce	ession	
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Hole Diameter Depth Metres Diameter		Cons	truction Recc	ord		Tes Pumping test method	st of Well Yield	 	
Hole Diameter Depth Metres Diameter From To Centimetres	Inside diam M	Cons	truction Recc Wall thickness	ord Depth	Metres	Tes Pumping test method	t of Well Yield Draw Down Time Water Leve	Re	ecovery Water Leve
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Hole Diameter Depth Metres Diameter From To Centimetres O 10-67 204 Water Record Water found Water found Kind of Water	Inside diam centimetres 8" Steel Plastic Galva Steel	Cons Aaterial Fibreglass ic Concrete anized Fibreglass	truction Recc Wall thickness centimetres Casing	Depth From	Metres To 10.67	Tes Pumping test method Pump intake set at - (metres) Pumping rate 56.4 (litres/min) 23 Duration of pumping hrs + 30 min	t of Well Yield Draw Down Time Water Leve min Metres Static Level 6.0	Time min 1	ecovery Water Leve Metres
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Hole Diameter Depth Metres Diameter From To Centimetres O 10-67 2-04 Water Record Water found at Metres Kind of Water m Fresh Sulphur Gas Salty Minerals Other:	Inside diam centimetres	Cons Aaterial Fibreglass ic Concrete anized Fibreglass ic Concrete anized	truction Reco Wall thickness centimetres Casing	Depth From	Metres To 10.67	Tes Pumping test method Pump intake set at - (metres) Pumping rate 56.4 (litres/min) 22 Duration of pumping hrs + 30 min Final water level end of pumping 8 metres Recommended pump type.	t of Well Yield Draw Down Time Water Leve min Metres Static Level 6.0 1 2 3 4	Time min 1 2 3 4	ecovery Water Leve Metres
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Hole Diameter         Depth       Metres       Diameter         From       To       Centimetres         O       O-67       2-044         Water Record       Water found       Kind of Water         m       Fresh       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Gas       Salty         m       Fresh       Sulphur         Gas       Salty       Minerals         Other:       Sulphur         Gas       Salty       Minerals         Other:       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Gas       Salty         Minerals       Other:       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Gas       Salty         Minerals       Other:       Sulphur       Sulphur         Gas       Salty       Minerals       Other:         After test of well yield, water was       Clear and sediment free	Inside diam centimetres	Cons Material Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized	truction Reco Wall thickness centimetres Casing .1 & & Solot No.	Depth From	Metres To 10.67	Tes         Pumping test method         Pumping test method         Pumping rate         Pumping	t of Well Yield Draw Down Time Water Leven min Metres Static Level 6. O 1 2 3 4 5 5 10 15 20 25 30	Rec Time min 1 2 3 4 5 5 10 15 20 25 30	Covery Water Leve Metres
Hole Diameter         Depth       Metres       Diameter         From       To       Centimetres         O       IO-67       IO-67       IO-67         Water found at       Kind of Water         Mater found at       Sulphur         Gas       Salty       Minerals         Other:       Sulphur         Gas       Salty       Minerals         Other:       Sulphur         Minerals       Sulphur         Gas       Salty       Minerals         Other:       Sulphur         After test of well yield, water was       Clear and sediment free         Other, specify       Other, specify	Inside diam centimetres	Cons Material Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized No C	truction Reco Wall thickness centimetres Casing .1 & & Slot No. Slot No.	Depth From	Metres To 10.67	Tes Pumping test method Pump intake set at - (metres) Pumping rate for the formation of pumping Intersection of pumping Intersection of pumping Recommended pump type. Shallow Deep Recommended pump depth. metres Recommended pump rate. (Intres/min) If flowing give rate - (Intres/min) If pumping discontin- ued, give reason.	t of Well Yield Draw Down Time Water Leve min Metres Static Level 6.0 1 2 3 4 5 10 15 20 25 30 40	Re Time min 1 2 3 4 5 5 10 15 20 25 30 40	ecovery Water Leve Metres
Hole Diameter         Depth       Metres       Diameter         From       To       Centimetres         O       O-67       2044         Water found at Metres       Kind of Water         m       Fresh       Sulphur         Gas       Salty       Minerals         Other:       m       Fresh       Sulphur         Gas       Salty       Minerals         Other:       m       Fresh       Sulphur         Gas       Salty       Minerals         Other:       After test of well yield, water was       Clear and sediment free         Other, specify       Chlorinated       Yes       No	Inside diam centimetres	Cons Material Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized No C	truction Reco Wall thickness centimetres Casing .1 & & Solot No. Slot No.	Depth From	Metres To 10.67	Tes         Pumping test method         Pumping test method         Pumping rate         Pumping	t of Well Yield Draw Down Time Water Leve min Metres Static Level 6. O 1 2 3 4 5 10 15 20 25 30 40 50 60 7. 01	Refinition in the second secon	Covery Water Level Metres
Hole Diameter         Depth       Metres       Diameter         From       To       Centimetres         O       IO-67       IO-67       IO-67         Water found at Metres       Kind of Water         Mater found at Other:       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Sulphur         Gas       Salty       Minerals         Other:       No       Minerals         Other:       No       Minerals         Plugging and S       Minerals	Inside diam centimetres	Cons Aaterial Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized No C hole	truction Reco Wall thickness centimetres Casing .1 8 8 Screen Slot No.	Depth From O	Metres To 10.67	Tes Pumping test method Pump intake set at - (metres) Pumping rate for the formation of pumping Interes Recommended pump	Image: store     Image: store       Draw Down       Time Water Leve       min     Metres       Static     6.0       1       2       3       4       5       10       15       20       25       30       40       50       60       7.01	Ref Time min 1 2 3 4 5 5 10 15 20 25 30 40 50 60	ecovery Water Leve Metres
Hole Diameter         Depth       Metres       Diameter         From       To       Centimetres         O       O-67       2004         Water Record       Water Record         Water found at Metres       Kind of Water         m       Fresh       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Gas       Salty         M       Fresh       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Gas       Salty         Minerals       Other:       Sulphur         Gas       Salty       Minerals         Other:       Sulphur       Gas       Salty         Gas       Salty       Minerals         Other:       Sulphur       Minerals         Gas       Salty       Minerals         Other:       Sulphur       Minerals         Chear and sediment free       Other, specify       No         Plugging and S       Material and t       Material and t	Inside diam centimetres	Cons Material Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized Fibreglass ic Concrete anized No C hole	truction Reco Wall thickness centimetres Casing Casing Siot No. Slot No.	Depth From C	Metres To 10.67	Tes         Pumping test method         Pumping test method         Pumping rate         Pumping         Prinal water level end         of pumping         metres         Recommended pump         depth         Recommended pump         rate         (litres/min)         If flowing give rate -         (litres/min)         If pumping discontinued, give reason.	t of Well Yield Draw Down Time Water Leve min Metres Static Level 6.0 1 2 3 4 5 10 15 20 25 30 40 50 60 7.01 of Well om road, lot line.	Ref Time min 1 2 3 4 5 5 10 15 20 25 30 40 50 60	ecovery Water Leve Metres

From To	aterial and type (bentonite slurry, r	neat cement slurry) etc	c. (cubic metres)	In diagram below show distances of well from r	oad, lot line, and building.
0 10 F	BENIONITE SL	NRRY		DEREY RD.	
				4	HWY25
				G.P.S.	10100
				1 1.20 20 560	62600
				N.40 29. 000	H VALANT FILLO
	Method of Cons	truction	1	W079 50.625 FT	P
Cable Tool	Rotary (air)	Diamond	Digging		
Rotary (conventional)	Air percussion	Jetting	Other	·	
Rotary (reverse)	Boring	Driving	······································	· ·	
	Water Use	<u>}</u>		BRITANIA RD	
		Public Supply	Other		
	Municipal	Cooling & air cc	onditioning		ell Completed
	Final Status o	fWell		2 039/6	103 12 1C
Water Supply	Recharge well	Unfinished	Abandoned, (Other)	Was the well owner's information Date De	livered YYYY MM DD
Observation well	Abandoned, insufficient supply	Dewatering		package delivered?	03 12 20
rest Hole	Mall Contractor/Technic	Replacement w	eli	Ministry Use Or	
Name of Well Contractor		Well'C	Contractor's Licence No.	Data Source Contrac	
S.D.SmilH	VKILLIDG CO.	<u> 10.</u> 2	+868		2000
RR#2	ACON OD	T LTJ	268	Date Received YYXX AMM DD Date of	Inspection YYYY MM DD
Name of Well Technician	(last name, first name)	WellTT	echnician's Licence No.	Remarks Well Re	cord Number
Signature of Techninian/C	Contractor	Date Su	Jebmitted YYYY MM DD	2	809874
0506E (09/03)	Contracto	r's Copy 📋 Minis	try's Copy 🗌 Well Owr	ner's Copy Cette formu	ile est disponible en françai

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Cette formule est disponible en français



		Tentapione										LO	G	JF	BO	KE	IOLE 1
Pro	ject I	No. : 1-22-0209-01	Clie	ent	: T	hom	as Rol	ert Colbec	ck							Origin	ated by :DH
Dat	e sta	rted : June 13, 2022	Pro	ject	: 6	360 I	Regior	al Road 2	5							Comp	oiled by :HR
She	eet N	o. :1 of 1	Loc	atic	on:N	/lilton	, Onta	rio								Cheo	ked by:MMT
Posi	tion	: E: 593189, N: 4816392 (UTM 17T)				Elevati	on Datu	n : Geodeti	с								
Rig	type	: Track-mounted				Drilling	Method	: Solid ste	em au	gers							1
Depth Scale (m)	Elev Depth (m)	Description	Braphic Log	Number	Type	PT 'N' Value	levation Scale (m)	(Blows / 0.3m) × Dynamic Co 10 2 Undrained She ○ Unconfined ● Pocket Per	ne 0 ar Stre 1 netromet	3 <u>0 40</u> ngth (kPa) + Field er ∎ Lab	) ) Vane Vane	Moisture Plastic N Limit Wate	/ Plastic atural r Content	Liquid Limit	Headspace Vapour (ppm)	Instrument Details	Lab Data and Comments GRAIN SIZE DISTRIBUTION (%) (MIT)
0	192.0	GROUND SURFACE		+		_⊡ 	Ш 192 -	40 8	0 1	20 16	0	10	20 3	30			GR SA SI CL
	191.2	(WEATHERED/DISTURBED), trace organics		1	SS	10							0				<u>SS1 Analysis:</u> M&I
·1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	SS	27	191 -					0					
·2				3	SS	31	190 -					0					
				4	SS	27						0					
•3				5	SS	30	189 -					0					
·4																	
	<u>187.4</u> 4.6	SANDY SILT to SAND AND SILT. trace	0			76/					$\setminus$						
5		to some clay, trace to some gravel, very dense, reddish brown, moist to wet (GLACIAL TILL)		6		275mm	187 -					0					
-6			<b>0</b>				186 -										
			6	7	SS	50 / 125mm	-					0					
7			<b>9</b>				185 -										
8			0.000 0.000000	8	SS	50 / 125mm	184 -					0					. 13 32 42 13
9	182.7			Ø	SS	50 /	183 -					0					

50 mm dia. monitoring well installed.

file: 1-22-0209-01 bh logs.gpj

	Terraprobe											I	_0	GC	<b>)</b> F	BO	REŀ	IOLE 2
Project I	No. : 1-22-0209-01	Clie	ent	: T	homa	as Rol	oert C	Colbe	ck								Origin	ated by :DH
Date sta	rted :June 17, 2022	Pro	ject	t :6	360 I	Regior	nal R	oad 2	5								Comp	oiled by :HR
Sheet N	o. :1 of 1	Loc	atic	on : N	/lilton	, Onta	rio										Cheo	ked by:MMT
Position	: E: 593164, N: 4816428 (UTM 17T)				Elevati	on Datu	m : (	Geodeti	с									
Rig type	: Track-mounted				Drilling	Method	: 8	Solid ste	em au	gers								
Ê	SOIL PROFILE			SAMP	LES	ale	Penet (Blows	ration Te s / 0.3m)	st Value	es		м	oisture /	Plasticit	tv	e	It	Lab Data
Depth Scale ( (m) (m)	Description	Sraphic Log	Number	Type	PT 'N' Value	Elevation Sca (m)	× D Undra O	ynamic Co 1 <u>0</u> 2 ined She Jnconfine Pocket Pe	one 0 3 ear Strei d netromet	30 4 ngth (kP + Fi er ■ La	4 <u>0</u> 'a) eld Vane ab Vane	Plasti Limit	c Nat Water	ural Content	Liquid Limit	Headspac Vapour (ppm)	Instrumer Details	Peneric and Comments Comments GRAIN SIZE DISTRIBUTION (%) (MIT)
-0 <b>192.6</b> 192.4	GROUND SURFACE	<u>x<sup>1</sup>/x</u>			S	ш.		10 8	0 1	20 1	60	1	0 2	) 30	)			GR SA SI CL
- 191.8	(WEATHERED/DISTURBED), trace organics		1	SS	12	192 -		$\mathbf{\Lambda}$					0					
-1	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist		2	SS	21		-						0					
-	(GLAGIAL TILL)		3	SS	30	191 -							0					
-2							-			$\left  \right\rangle$								
-3			4		30	190 -							0					
			5	SS	34								o					
-4							-											
-			6	SS	38	188 -							0					
-5							-											
						187 -					$\left  \right\rangle$							
-6 <u>186.5</u> 6.1	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very	0	7	SS	50 / 125mm	· .							0					
-	dense, moist to wet (GLACIAL TILL)	¢				186 -												
		0					-											
-8		0	8	<u>ss</u>	50 / 100mm	185 -						0						
		0				404												
-9 <u>183</u> .4		9			. 50 /	104 -									0			
9.2			ت		75mm													

Borehole was dry and open upon completion of drilling.

		Terraprobe							LOG OF	BOREI	HOLE 3
Proj	ect N	lo. : 1-22-0209-01	Clie	ent	: T	homa	as Rob	pert Colbeck		Origin	ated by :DH
Dat	e sta	rted :June 17, 2022	Pro	ject	: : 6	360 I	Region	nal Road 25		Com	piled by:HR
She	et No	o. :1 of 1	Loc	catio	on : N	/lilton	, Ontai	rio		Che	cked by :MMT
Posit	ion	: E: 593178, N: 4816411 (UTM 17T)				Elevati	, on Datur	m : Geodetic			,
Rig t	/pe	: Track-mounted			I	Drilling	Method	: Solid stem augers			
٦ آ		SOIL PROFILE			SAMPI	LES	ale	Penetration Test Values (Blows / 0.3m)	Moisture / Plasticity	e e	Lab Data
Depth Scale (r	<u>Elev</u> Depth (m)	Description	Graphic Log	Number	Type	sPT 'N' Value	Elevation Sca (m)	X Dynamic Cone 10 20 30 40 Undrained Shear Strength (kPa) O Unconfined + Field Vane ◆ Pocket Penetrometer ■ Lab Vane ◆ 0 90 100 100 100	Plastic Natural Liquid Limit Water Content Limit	Headspac Vapour (ppm) Instrumer Details	Performance Zijareston Distribution (%) (MIT)
0	192.5		<u>x 1/4.</u>			0		40 80 120 180	10 20 30		GR SA SI CL
	191.7	(WEATHERED/DISTURBED), trace organics		1	SS	11	192 —		0	PID: 0 FID: 0	<u>SS1 Analysis:</u> M&I
1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	SS	25	- 101		0	PID: 0 FID: 0	
2				3	SS	29	-		0	PID: 0 FID: 0	
3				4	SS	26	190 —		0	PID: 0 FID: 0	
0				5	SS	35	189 —		0	PID: 0 FID: 0	<u>SS5 Analysis:</u> .pH
4							- 188				
-5				6	SS	36	-		0	PID: 0 FID: 0	
· 6	186.4						187 —				
	6.1	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist to wet (GLACIAL TILL)	¢	7	SS	66	186 —		0	PID: 0 FID: 0	
.7			0	[∞]	SS	50 / 125mm	- 185 -		0	PID: 0 FID: 0	
			ø				184 —				
9	183.3 9.2			9	SS	50 / 100mm	-			PID: 0 FID: 0	

Borehole was dry and open upon completion of drilling.

	<b>X</b>	Terraprobe								LO	G C	)F	BO	REF	IOL	E 4	ŀ
Pro	ject l	No. : 1-22-0209-01	Clier	nt	: Tho	oma	s Rob	ert Colbeck						Origina	ated by	: DH	ł
Dat	e sta	rted :June 17, 2022	Proje	ect	: 636	30 R	Region	al Road 25						Comp	iled by	: HF	ł
She	et N	o. :1 of 1	Loca	atior	า : Milt	ton,	Ontar	io						Chec	ked by	: MN	ЛТ
Posi	tion	: E: 593136, N: 4816448 (UTM 17T)			Ele	vatio	on Datur	m : Geodetic									٦
Rig t	уре	: Track-mounted			Dril	Iling I	Method	: Solid stem augers									
E)	<u> </u>	SOIL PROFILE			AMPLES	<u>9</u>	cale	(Blows / 0.3m)		Moisture	/ Plasticit	y	, r	ent s	La v =	ab Data and	
Depth Scale	Elev Depth (m)	Description	sraphic Lo	Number	Type	⊃T 'N' Valu	levation S (m)	10 20 30 4( Undrained Shear Strength (kPa O Unconfined + Fiel ● Pocket Penetrometer ■ Lab	0 I) Id Vane Vane	Plastic Na Limit Water	itural l Content	Liquid Limit	Headsp. Vapou (ppm	Instrum Detail	Unstabilize Water Leve	RAIN SIZE RIBUTION (MIT)	3 (%)
-0	192.6	GROUND SURFACE	<u> </u>	+		<u>க்</u>	Ш	40 80 120 16	60	10 2	20 30	)			G	RSASI	CL
		(WEATHERED/DISTURBED)		1	SS	4	192 —				0						
·1	<u>191.8</u> 0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	ss 2	22	_			0							
-2				3	ss 2	26	191 —			0							
2				4	ss :	29	- 190			0							
-3				+	_		-										
				5	ss a	34	189 —			0							
- 4							-										
- 5				6	ss a	36	- 100			0					2	19 53	26
- 6	<u>186.5</u> 6.1			_		50 /	187 —										
		some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	Q	7	SS 150	Jmm	186 —			0			•				
-7			ø				_						-				
- 8		wet below	00000000000000000000000000000000000000	8_	<u>SS 5</u> 100	i0 / 0mm	185 —			0			- - - - -				
- 9			0				184 —										
	183.3	<u> </u>		9	SS 5	i0 / 5mm	-			0					<b>I</b>		_
	0.0	END OF BOREHOLE Borehole was dry and open upon			μ <u>ζ</u>	<u></u>		WAT <u>Date</u> Jul 14, 2022	ER LE <u>Water</u>	VEL READIN Depth (m) 6.8	GS <u>Elevati</u> 18	<u>ion (m</u> 5.8	Ď				

50 mm dia. monitoring well installed.

file: 1-22-0209-01 bh logs.gpj

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Terraprobe
Terraprobe

## LOG OF BOREHOLE 5

Originated by : DH Compiled by : HR

Checked by : MMT

Date started : June 13, 2022

: 1-22-0209-01

:1 of 1

Project No.

Sheet No.

Project : 6360 Regional Road 25

: Thomas Robert Colbeck

Location : Milton, Ontario

Client

Posit	ion	: E: 593158, N: 4816475 (UTM 17T)			I	Elevati	on Datu	m : G	Beodeti	с									
Rig t	ype	: Track-mounted				Drilling	Method	: S	olid ste	em aug	gers								
Ê		SOIL PROFILE			Sampi	ES	lle	Penetr (Blows	ation Te / 0.3m)	st Value	s		м	oisture	Plastici	tv	ė	ıt	Lab Data
Depth Scale (r	Elev Depth (m) <b>192.6</b>	Description GROUND SURFACE	Graphic Log	Number	Type	SPT 'N' Value	Elevation Sca (m)	× Dy 1 Undrai O U ● F 4	namic Co 0 2 ned She Inconfined Pocket Per 0 8	ne <u>03</u> ar Strer i netromete 01	0 4 gth (kPa + Fie r∎ Lal 20 16	0 a) Id Vane b Vane 50	Plasti Limit F	c Na Water	tural Content	Liquid Limit - 0	Headspac Vapour (ppm)	Instrumen Details	Balling Bal
Γ		100mm TOPSOIL					-	-											
-	191.8	FILL, clayey silt, trace gravel, trace sand, trace organics, compact, dark brown, moist		1	SS	13	192 –		$\backslash$					0					
-1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist		2	SS	24	-							0					
-				3	SS	37	191 –							0					
-2						20	-												
-3				4	55	30	190 -							0					
Ļ				5	SS	29	-							0					
-4							-												
-	<u>188.0</u> 4.6	SANDY SILT to SAND AND SILT, trace	1	6	SS	50 /	188 -					$\square$	0						
-5		to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	0			Toomin	-	-											
ŀ							187												
-6						50 /	-	-											
-			9			125mm	186 -												
-7			0				-	-											
-		wet	9	8	55	71/	185 —							0					
-8			9			250mm	-												
-			9				184 -												⊻
-9			9		99	50/	-												
I I	183.2 9.4				00	75mm								1					

END OF BOREHOLE

Unstabilized water level measured at 8.8 m below ground surface; borehole was open upon completion of drilling.

		Terraprobe							LOG OF	BOF	REF	IOLE 6
Proj	ect N	lo. : 1-22-0209-01	Clie	ent	: ٦	homa	as Rob	pert Colbeck			Origina	ated by :DH
Date	e sta	rted :June 14, 2022	Pro	jec	t :6	360 I	Regior	nal Road 25			Comp	oiled by :HR
She	et No	p.: 1 of 1	Loc	atio	on : N	/ilton	, Onta	rio			Chec	ked by:MMT
Posit	ion	: E: 593175, N: 4816456 (UTM 17T)				Elevati	ion Datu	m : Geodetic				
Rig ty	/pe	: Track-mounted				Drilling	Method	I : Solid stem augers				
Ê		SOIL PROFILE			SAMP	LES	ale	Penetration Test Values (Blows / 0.3m)	Moisture / Plasticity	e	ц	Lab Data
Depth Scale (	<u>Elev</u> Depth (m)	Description	Graphic Log	Number	Type	PT 'N' Value	Elevation Sc (m)	× Dynamic Cone <u>10</u> 20 30 40 Undrained Shear Strength (kPa) O Unconfined + Field Vane ● Pocket Penetrometer ■ Lab Vane	Plastic Natural Liquid Limit Water Content Limit	Headspa Vapour (ppm)	Instrume Details	Parality Comments Comments GRAIN SIZE DISTRIBUTION (%) (MIT)
-0	192.7		/ XXX	ź		05		40 80 120 180				GR SA SI CL
-	191.9	FILL, clayey silt, trace gravel, trace sand, trace organics, compact, dark brown, moist	′ 💥	1	SS	9	- 192 -		0	_PID: 0 FID: 0		<u>SS1 Analysis:</u> M&I
-1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	SS	16	-		0	_PID: 0 FID: 0		
-				3	SS	24	191 –		0	PID: 0 FID: 0		
-				4	SS	28	-		Ф	_PID: 0 FID: 0		
							190 -					
-3				5	SS	34	-   -		0	_PID: 0 FID: 0		
-4							189 -					
-	188.1						-					
-5	4.0	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	0	6	SS	72 / 275mm	188 -		0	_PID: 0 FID: 0		<u>SS6 Analysis:</u> .pH
-			Φ				187 -					
-6		sandy silt, trace clay, wet	9	7	SS	50 / 100mm	-		0	_PID: 0 FID: 0		
-			0				186 -					
Ĺ			0									
-8				8	SS	50 / 125mm	185 -		Φ	_PID: 0 FID: 0		Ţ
-			<del>.</del>				184 -					
-9	183.3 9.4	wet		9	SS	50 / 75mm	-   -		0	_PID: 0 FID: 0		

Unstabilized water level measured at 7.9 m below ground surface; borehole caved to 7.9 m below ground surface upon completion of drilling.

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		Terraprobe						LO	G OF BC	REHOLE 7
Pro	ject N	No. : 1-22-0209-01	Clie	nt	: T	homa	as Rol	ert Colbeck		Originated by : DH
Dat	e sta	rted : June 14, 2022	Proj	ect	: 6	360 F	Regior	al Road 25		Compiled by : HR
She	et N	o. :1 of 1	Loca	atio	on:N	lilton	, Onta	0		Checked by : MM
Posi	tion	: E: 593197, N: 4816435 (UTM 17T)				Elevati	on Datu	: Geodetic		,
Rig t	уре	: Track-mounted				Drilling	Method	: Solid stem augers		
(E		SOIL PROFILE		5	SAMP	LES	ale	Penetration Test Values (Blows / 0.3m) Moisture /	Plasticity 8	Lab Data
epth Scale	Elev Depth (m)	Description	aphic Log	Number	Type	T 'N' Value	evation Sc (m)	× Dynamic Cone 10 20 30 40 Undrained Shear Strength (kPa) O Unconfined PledVane PledVane	Headspart Content Timit Capour Headspart Heads	Comments transition transiti
-0	192.7	GROUND SURFACE	Ū			Ъ		40 80 120 160 10 20	5 30	GR SA SI C
		125mm <b>TOPSOIL</b> (WEATHERED/DISTURBED), trace organics		1	SS	10		0		
• 1	<u>191.9</u> 0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist		2	SS	20	192 -	0		
		(GLACIAL TILL)					-			
-2				3	SS	26	191 -	o		
				4	SS	32	190 -	o		
- 3				5	SS	31		0		
- 4							189 -			
	<u>188.1</u> 4.6						-			
- 5		to some clay, trace to some gravel, dense to very dense, reddish brown, moist (GLACIAL TILL)	<u></u>	6	SS	41	- 100	0		
- 6			9				187 -			
		silty sand lense, wet	0	7	SS	50 / 150mm		c		
-7			0				- 100			
- 8			0	8	SS	50 / 125mm	185 -	0		
			<u>.</u>				184 -			
· 9	183.3	wet	9	9	SS	50 / 100mm		•	)	
	3.4	END OF BOREHOLE						WATER LEVEL READINO <u>Date</u> <u>Water Depth (m)</u> Jul 14, 2022 7.2	GS <u>Elevation (m)</u> 185.5	

50 mm dia. monitoring well installed.

file: 1-22-0209-01 bh logs.gpj

Terraprobe

: 1-22-0209-01

## **LOG OF BOREHOLE 8**

Originated by : DH

Compiled by : HR

Date started : June 14, 2022

Project No.

Project : 6360 Regional Road 25

: Thomas Robert Colbeck

Client

She	et No	p. :1 of 1	Loc	atio	on : N	lilton	, Onta	io										Cheo	ked by:MMT
Posit	ion	: E: 593216, N: 4816419 (UTM 17T)			I	Elevati	on Datu	n : Geo	odetio	2									
Rig t	ype	: Track-mounted			I	Drilling	Method	: Soli	id ste	em aug	ers								
Ê		SOIL PROFILE			SAMPI	ES	le	Penetratio (Blows / 0	on Tes ).3m)	st Value	s		N	loisture	/ Plastic	itv	Ð	t	Lab Data
Depth Scale (n	Elev Depth (m) <b>192.6</b>	Description GROUND SURFACE	Graphic Log	Number	Type	SPT 'N' Value	Elevation Sca (m)	X Dynar 10 Undrained O Unco Pock 40	mic Cor 2( d Shea onfined ket Pen 8(	ne <u>) 3</u> ar Stren etromete ) 12	) ∠ gth (kP + Fid r ■ La 20 1	40 a) eld Vane ab Vane 60	Plast Limit	ic N Wate	atural r Content MC I 20 3	Liquid Limit Limit	Headspac Vapour (ppm)	Instrumen Details	Being and Comments Comments GRAIN SIZE DISTRIBUTION (%) (MIT) GR SA SI CL
- <sup>0</sup>		90mm TOPSOIL		XXI.			-												
╞	191.8	FILL, clayey silt, trace gravel, trace sand, trace organics, compact, dark brown, moist		1	SS	11	192 –								0				
- 1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist		2	ss	28	-							0					
-		(GLACIAL HEL)		3	SS	33	191 –							0					
-2						24	-												
				4		34	190 –												
				5	SS	35	-							0					
-4							189 -												
-	188.0			1			188 -					$  \rangle$							
-5	4.0	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	9	6	SS	79	-						0						
-			Ø				187 —												
-6		silty sand lense, wet	0	7	SS	84 / 250mm	-								0				
-7			<b>O</b>				186												
-		wet below	•	8	SS	50/	185 –							0					⊻
-8			0			125mm	-												
ŀ			0				184 –												
-9	183.3		10	9	SS	50 /	-						0						

END OF BOREHOLE

Unstabilized water level measured at 7.6 m below ground surface; borehole caved to 7.9 m below ground surface upon completion of drilling.

		Terraprobe							LOG OF	BORE	HOLE 9
Pro	ject N	No. : 1-22-0209-01	Clie	nt	: T	homa	as Rob	pert Colbeck		Origin	ated by:DH
Dat	e sta	rted :June 15, 2022	Proj	ect	: 6	360 F	Region	nal Road 25		Com	piled by :HR
She	et No	o. :1 of 1	Loca	atio	n:N	lilton	, Ontai	rio		Che	cked by:MMT
Posi	tion	: E: 593172, N: 4816498 (UTM 17T)			F	Elevati	on Datur	m : Geodetic			
Rig t	ype I	: Track-mounted		0	]	Drilling	Method	: Solid stem augers			1
epth Scale (m)	Elev Depth (m)	Description	aphic Log	Number	Type	T 'N' Value	evation Scale (m)	X Dynamic Cone     10     20     30     40     Undrained Shear Strength (kPa)     O Unconfined     Field Vane     Porket Benetrometer     In Vane	Moisture / Plasticity Plastic Natural Liquid Limit Water Content Limit	Headspace Vapour (ppm) Instrument Details	Lab Data and Comments Base Base Base Comments Base Base Comments C
-0	192.5	GROUND SURFACE	Ū			SP	Ū	40 80 120 160	10 20 30		GR SA SI CL
		FILL, clayey silt, trace gravel, trace sand, trace organics, stiff, dark brown, moiet		1	SS	12	192 —		0	_PID: 0 FID: 0	<u>SS1 Analysis:</u> M&I
- 1	<u>191.7</u> 0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	SS	21	-		0	_PID: 0 FID: 0	
-2				3	SS	37	191 —		0	_PID: 0 _FID: 0	
				4	SS	25	190 —		0	_PID: 0 FID: 0	
- 3	189.5 3.0	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	e V	5	SS	62	- 189 —		Ф	_PID: 0 _FID: 0	<u>SS5 Analysis:</u> PHC
- 5			<b>6</b>	6	SS	50 / 125mm	188 —		0	_PID: 0 FID: 0	
6			<del>.</del>				187 —				
0		wet below	0	7	SS	50 / 150mm	186 —		o	PID: 0 FID: 0	SS7 Analysis: PCB
-7		and and ait lance	9	0		50 /	- 185 <del>-</del>			PID: 0	
- 8		sano ano siù lense		8	_ 55	150mm	- 184 —			TFID: 0	
-9	<u>183.3</u> 9.2		0	<u> </u>	SS	50 /	-		0		
	0.2	END OF BOREHOLE Borehole was dry and open upon completion of drilling.				UUUUU		WATER LE\ <u>Date</u> <u>Water</u> Jul 11, 2022 Jul 14, 2022	/EL READINGS Depth (m) Elevation (n 6.5 186.0 6.8 185.7	<u>D</u>	

50 mm dia. monitoring well installed.

m

		Terraprobe											L	OG	O	FE	BOR	EH	OLE 10
Pro	ject I	No. : 1-22-0209-01	Clie	ent	: T	hom	as Rol	oert C	olbec	k								Origin	ated by :DH
Dat	e sta	rted :June 16, 2022	Pro	ject	t:6	360 I	Regior	nal Ro	ad 2	5								Com	piled by :HR
She	et N	o. :1 of 1	Loc	Location : Milton, Ontario														Che	cked by:MMT
Posi	tion	: E: 593184, N: 4816470 (UTM 17T)				Elevati	ion Datu	m :G	eodeti	C									
Rig t	ype	: Track-mounted		-		Drilling	Method	: S	olid ste	em au	gers								1
Depth Scale (m)	Elev Depth (m)	SOIL PROFILE Description GROUND SURFACE	Graphic Log	Number	SAMP adv	SPT 'N' Value	Elevation Scale (m)	Penetra (Blows X Dyr 1! Undrair O U • Po	ation Tes / 0.3m) namic Cor 0 2( ned She nconfined ocket Pen 0 8(	ne 0 3 ar Strer letromete	ag0 4 ngth (kP + Fi er ■ La 20 1	4 <u>0</u> a) eld Vane ab Vane 60	N Plast Limit	loisture / ic Nater Water PL M L 2	Plasticit	Liquid Limit	Headspace Vapour (ppm)	Instrument Details	Lab Data and Comments GRAIN SIZE DISTRIBUTION (%) (MIT)
-0	132.5	90mm TOPSOIL	/ 👹	*				Í	-							·			GR 3A 31 CL
-	192.1	FILL, clayey silt, trace gravel, trace sand, trace organics, stiff, dark brown, moist		1	SS	11	-		$\setminus$								FID: 0		<u>SS1 Analysis:</u> M&I
-1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	SS	15	192 -							0			_PID: 0 FID: 0		<u>SS2 Analysis:</u> PHC
-2				3	SS	28	191 -			$\rightarrow$				0			_PID: 0 FID: 0		
-				4	SS	26	-	-						0			_PID: 0 FID: 0		
-3 -				5	SS	30	. 190 -							0			_PID: 0 FID: 0		3 31 44 22 <u>SS5 Analysis:</u> .pH
-4							189 -												
- 5				6	SS	35	188 -							0			PID: 0 FID: 0		<u>SS6 Analysis:</u> PHC
-								-											
-6	<u>186.8</u> 6.1	SANDY SILT to SAND AND SILT trace	0	7	22	50 /	187 -										_PID: 0		
-		to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	9 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			125mm	186 -										FID: 0		· · ·
-7			<del>Q</del>																
-8		wet below	0	8	SS	50 / 125mm	185 -							0			_PID: 0 FID: 0		· • • •
-9	183.6		9	9	SS	50 /	184 -						(				PID: 0		
	9.3	END OF BOREHOLE Borehole was dry and open upon completion of drilling.				USUMM	J			<mark>Da</mark> Jul 11, Jul 14,	WA <u>te</u> 2022 2022	TER LE <u>Wate</u>	EVEL F e <b>r Dept</b> 8.0 7.1	Readin( h_(m)	GS <u>Elevat</u> 18 18	<u>ion (n</u> 4.9 5.8	<u>n</u> )		

50 mm dia. monitoring well installed.

file: 1-22-0209-01 bh logs.gpj

		Terraprobe											L	OG	OF E	BOR	EH	OLE 11	
Proj	ect N	lo. : 1-22-0209-01	Clie	nt	: T	homa	as Rob	oert C	olbed	k							Origin	ated by :DH	
Date	e sta	rted :June 15, 2022	Pro	Project : 6360 Regional Road 25													Compiled by : HR		
She	et No	p.: 1 of 1	Loc	atio	on : N	/lilton.	Onta	rio									Cheo	cked by :MMT	
Posit	ion	: E: 593207, N: 4816449 (UTM 17T)				Elevatio	on Datu	m : G	eodeti	0								,	
Rig t	/pe	: Track-mounted			l	Drilling	Method	: Se	olid ste	em aug	jers								
Ê		SOIL PROFILE	1		SAMPI	ES	ale	Penetra (Blows	ation Te: / 0.3m)	st Value	s		м	loisture / F	lasticity	ø	t	Lab Data	
Depth Scale (r	<u>Elev</u> Depth (m)	Description	Graphic Log	Number	Type	sPT 'N' Value	Elevation Sca (m)	X Dyr 1( Undrair 0 Ui Pe	namic Col 0 2 ned She nconfined ocket Per	ne 2) 3 ar Stren	0 4 gth (kPa + Fie r ■ La	0 a) eld Vane b Vane	Plasti Limit	ic Natur Water Co PL MC	al Liquid Intent Limit	Headspac Vapour (ppm)	Instrumer Details	Point         and           Distribution         Comments           Distribution         GRAIN SIZE           DISTRIBUTION (%) (MIT)         (%)	
-0	192.5		/ ***	-		05		40	0 0	J 12	20 11	30		0 20	30			GR SA SI CL	
-	191.7	FILL, clayey silt, trace gravel, trace sand, trace organics, stiff, dark brown, moist		1	SS	9	192 –							0		-		<u>SS1 Analysis:</u> M&I	
-1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)		2	SS	22	-							0					
-2				3	SS	24	- 191 –							0					
-				4	SS	27	190 –			+				0		-			
-3 -				5	SS	30	- 189 -							0					
-4							-												
-5	4.6	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist	- 1	6	SS	74	- 188						С						
-		(GLACIAL TILL)	9 0	- - - - - - - - - - - - - - - - - - -			187 –									_			
-6		wet	0	7	SS	82 / 250mm	-							0					
-							186 —												
-			•				- 185									-			
-8		wet below	0	8	SS	50 / 100mm	-											⊻	
-			0	· · · · · · · · · · · · · · · · · · ·			184 –									_			
-9	183.2 9.3		0	9	SS	50 / 125mm	_						0						

END OF BOREHOLE

Unstabilized water level measured at 7.9 m below ground surface; borehole caved to 8.8 m below ground surface upon completion of drilling.

		Terraprobe							LOG OF B	BOR	EHOLE 12
Proj	ject I	No. : 1-22-0209-01	Clie	nt	: T	homa	as Rol	bert Colbeck			Originated by :DH
Date	e sta	rted : June 14, 2022	Pro	jec	t:6	360 I	Regior	nal Road 25			Compiled by : HR
She	et N	o 1 of 1	l oc	atio	on ∙ N	/ilton	Onta	ario			Checked by · MMT
Posit	ion	· E· 593228 N· 4816434 (UTM 17T)	200		, iii	Flevati	on Datu	im : Geodetic			
Ria t	vpe	: Track-mounted				Drillina	Method	d : Solid stem augers			
		SOIL PROFILE			SAMPI	LES	e	Penetration Test Values	Maiatura / Diastiaitu	0	Lab Data
Depth Scale (m	<u>Elev</u> Depth (m)	Description	traphic Log	Number	Type	PT 'N' Value	levation Scal (m)	X Dynamic Cone           10         20         30         40           Undrained Shear Strength (kPa)         O         Unconfined         + Field Vane           ● Pocket Penetrometer         ■ Lab Vane         Nane	Moisture / Plasticity Plastic Natural Liquid Limit Water Content Limit PL MC LL	Headspace Vapour (ppm)	and Comments
-0	192.5		0	_		5	ш	40 80 120 160	10 20 30		GR SA SI CL
-		(WEATHERED/DISTURBED)		1	SS	11	192 -		0		
-1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist		2	SS	15			0		
-		(GLACIAL TILL)		3	SS	22	. 191 -		0		
-2					22	40	190 -				
-3									0		
-				5	SS	31	189 -		0		
-4							188 -				
-5				6	SS	19			0		
-	100.4						187 -				
-	6.1	SAND, trace silt, trace clay, very dense, brown, wet		7	SS	26 / 250mm	186 -		0		
-7							- 185 -				
-8	184.9 7.6	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, wet (GLACIAL TILL)		8	SS	50 / 150mm		-	0		¥
- 9			9				184 -				
ľ	<u>183.1</u> 9.4		0	9	SS	50 / 100mm			0		

Unstabilized water level measured at 7.6 m below ground surface; borehole caved to 8.8 m below ground surface upon completion of drilling.

file: 1-22-0209-01 bh logs.gpj

		Terraprobe							LOG OF E	BORE	IOLE 13		
Proj	ect N	Jo. : 1-22-0209-01	Clier	nt	: Th	ioma	as Rob	ert Colbeck		Orig	inated by :DH		
Date	e sta	rted : June 16, 2022	Proj€	əct	: 63	60 F	Region	al Road 25	Со	Compiled by : HR			
She	et No	o. :1 of 1	Loca	atior	ו: Mi	lton,	Ch	Checked by : MMT					
Posit	ion	: E: 593188, N: 4816515 (UTM 17T)			El	levatio	on Datur	n : Geodetic					
Rig ty	/pe	Track-mounted			Dr	rilling	Method	: Solid stem augers		<u> </u>			
Depth Scale (m)	Elev Depth (m)	Description	Braphic Log	Number	Type	PT 'N' Value	levation Scale (m)	X Dynamic Cone     10     20     30     40       Undrained Shear Strength (kPa)     0     Unconfined     + Field Vane       • Pocket Penetrometer     Lab Vane	Moisture / Plasticity Plastic Natural Liquid Limit Water Content Limit	Headspace Vapour (ppm) Instrument	Lab Data and Data Comments GRAIN SIZE DISTRIBUTION (%) (MIT)		
-0	192.4	GROUND SURFACE		+	$\rightarrow$	ю.	ш	40 80 120 160	10 20 30		GR SA SI CL		
	191.6	FILL, clayey silt, trace gravel, trace sand, trace organics, stiff, dark brown, moist	-	1	ss	10	192 —			_PID: 5 FID: 0	<u>SS1 Analysis:</u> M&I		
- 1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff, brown to reddish brown, moist (GLACIAL TILL)		2	ss	22	- 191 —		0	_PID: 0 FID: 0			
-2				3	ss	27	_		0	_PID: 0 FID: 0			
- 3	189.4			4	ss	27	190 —		0	_PID: 0 FID: 0			
-	3.0	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	<b>0</b>	5	ss	50	189 —		o	_PID: 0 FID: 0			
- 4			0.000	6	SS 1	50 / 25mm	188 —		0	_PID: 0 FID: 0			
- 5							187 —						
- 6			<b>₽</b>	7	SS 1	50 / 50mm	- 186 —		0	PID: 0	8 36 43 13		
-7			0				- 185 -						
-8		wet below	0	8	ss tu	50 / 00mm	-		0	_PID: 0	SS8 Analysis: PCB, PHC		
- 9			<b>.</b>				184 — -						
	183.1			9	SS 1	50 / 50mm			0	PID: 0 FID: 0			
	5.5	END OF BOREHOLE Borehole was dry and open upon completion of drilling.						WATER LE' <u>Date</u> <u>Water</u> Jul 11, 2022 Jul 14, 2022	VEL READINGS <u>Depth (m) Elevation (n</u> 6.7 185.7 6.8 185.6	ŋ			

50 mm dia. monitoring well installed.



	3	Terraprobe										L	OG	6 O	FE	BOR	EH	OLE 14
Proje	ect N	lo. : 1-22-0209-01	Clie	ent	: ٦	Thom	as Rol	ert Colb	eck								Origin	ated by :DH
Date	e sta	rted : June 16, 2022	Proj	jec	t :6	6360	Regior	al Road	25			Compiled by :HR						
Shee	et No	o. :1 of 1	Loc	atio	on : N	Milton	, Onta	rio				Checked by : MMT						cked by :MMT
Positi	on	: E: 593207, N: 4816494 (UTM 17T)				Elevati	ion Datu	m : Geod	etic									
Rig ty	pe	: Track-mounted				Drilling	Method	: Solid	stem au	ugers	-							
(E)		SOIL PROFILE	-		SAMP	LES	ale	Penetration (Blows / 0.3	Test Valı m)	ies	-	N	Moisture / Plasticity			ee	nt	Lab Data
)epth Scale (	<u>Elev</u> Depth (m)	Description	iraphic Log	Number	Type	P. Value	levation Sca (m)	X Dynamic 10 Undrained S O Unconf Pocket	Cone 20 Shear Stre ined Penetrome	3,0 ength (kl + F	40 Pa) Field Vane Lab Vane	Plast – Limit	ic Nate	atural r Content MC	Liquid Limit	Headspac Vapour (ppm)	Instrumeı Details	B and Comments Comments GRAIN SIZE DISTRIBUTION (%) (MIT)
-0	192.2	GROUND SURFACE	<u></u>			L S	ш	40	80	120	160		10 2	20 :	30			GR SA SI CL
-	191.4	(WEATHERED/DISTURBED), trace organics		1	ss	9	192 -						0					
-1	0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff, brown to reddish brown, moist (GLACIAL TILL)		2	SS	20	191 -											
-				3	ss	23							0					
-2							190 -											
-				4	ss	29							0					
-3	189.2 3.0		6	]			180 -				$\mathbf{N}$							
-		to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)	0	5	SS	60	109-					0						
-4			0				188 -											
5			0	6	SS	50 / 150mm	- 1					o						
-			9				187 -											
-6 -			0	7	SS	50 / 125mm	186 -					0						
-7			0				185 -											
- 			6	8	SS	50 / 75mm	184 -					0						
-9	<u>183.0</u> 9.2	END OF BOREHOLE	0	9	SS	50 / 100mm	183 -					0						

		Terraprobe						LOG OF E	BOREHOLE 15
Pro	ject N	lo. : 1-22-0209-01	Client	: T	homa	as Rob	pert Colbeck		Originated by :DH
Dat	e stai	ted :June 16, 2022	Project	t:6	360 F	Regior	nal Road 25		Compiled by : HR
She	et No	o. :1 of 1	Locatio	on:N	lilton	, Onta	rio		Checked by :MMT
Posi	tion :	E: 593225, N: 4816475 (UTM 17T)			Elevati	on Datu	m : Geodetic		
Rig t	ype :	Track-mounted			Drilling	Method	: Solid stem augers		
pth Scale (m)	Elev Depth	SOIL PROFILE	Aphic Log Jumber	Type	'N' Value	vation Scale (m)	Penetration 1 est Values (Blows / 0.3m) X Dynamic Cone 10 20 30 40 Undrained Shear Strength (kPa) O Unconfined + Field Vane	Moisture / Plasticity Plastic Natural Liquid Limit Water Content Limit	Headsbace A apour Capour (ppm)) Headsbace (ppm)) Headsbace (ppm)) Headsbace (ppm)) Headsbace (ppm)) Headsbace (ppm)) Headsbace (ppm)) (ppm)) (ppm)) (ppm))
മ് - റ	192.2	GROUND SURFACE	5 5		SP1	Ее	Pocket Penetrometer ■ Lab Vane 40 80 120 160	10 20 30	(MIT) GR SA SI CL
U		115mm TOPSOIL FILL, clayey silt, trace gravel, trace sand, trace organics, stiff, brown, moist	1	SS	8	192 –		0	-PID: 0 <u>SS1 Analysis:</u> M&I
- 1	<u>191.4</u> 0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddish brown, moist (GLACIAL TILL)	2	SS	26	- 191		0	-PID: 0
-2			3	SS	24	- 190 –		0	-PID: 0
- 3			4	SS	27	-		0	-PID: 0 <u>SS4 Analysis:</u> .pH
			5	SS	28	189 -		0	-PID: 0
- 4						188 -			
- 5			6	SS	31	187 –		0	
- 6	<u>186.1</u> 6.1	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, wet	<b>6</b> 7	SS	83 / 275mm	186 -		0	-PID: 0
-7		(GLACIAL TILL)				- 185 –			
- 8			8 8	<u>ss</u>	50 / 75mm	- 184 –		0	-PID: 0
- 9	<u>182.9</u> 9.3		9	SS	50 / 125mm	183 –		0	-PID: 0
		END OF BOREHOLE Borehole was dry and open upon					WATER LEV <u>Date</u> <u>Water</u> Jul 11, 2022 Jul 14, 2022	/EL READINGS Depth (m) Elevation (n 7.0 185.2 7.1 185.1	ח

completion of drilling.



		Terraprobe							LOG OF E	<b>SOREHOLE 16</b>				
Proj	ect I	No. : 1-22-0209-01	Clie	nt	: T	hom	as Rot	oert Colbeck		Originated by : DH				
Date	e sta	rted : June 15, 2022	Proj	ject	: 6	i360 I	Regior	nal Road 25	Compiled by :HR					
She	et N	o. :1 of 1	Loc	atio	on : N	<i>l</i> ilton	. Onta	rio	Checked by : MMT					
Posit	ion	: E: 593246, N: 4816455 (UTM 17T)				Elevati	on Datu	m : Geodetic		-				
Rig t	уре	: Track-mounted			!	Drilling	Method	: Solid stem augers						
th Scale (m)	<u>Elev</u>	SOIL PROFILE Description	hic Log	mber	SAMPL 	N' Value	ation Scale (m)	Penetration Test Values (Blows / 0.3m) X Dynamic Cone 10 20 30 40 Undrained Shear Strength (kPa)	Moisture / Plasticity Plastic Natural Liquid Limit Water Content Limit	Capour cadspace (ppm) (p				
Dept	(m)		Grap	Ž	-	SPT "	Eleva	O Unconfined + Field Vane ● Pocket Penetrometer ■ Lab Vane 40 80 120 160	PL MC LL 10 20 30					
- 0	192.2	125mm TOPSOIL (WEATHERED/DISTURBED), trace organics		1	SS	12	192-			PID: 0 FID: 0 M&I				
- 1	<u>191.4</u> 0.8	CLAYEY SILT, some sand to sandy, trace gravel, very stiff to hard, brown to reddieb brown moiet	-	2	ss	26	-		0	LPID: 0 FID: 0				
		(GLACIAL TILL)					191 –			PID: 0				
-2					>> 	20	190 –			FID: 0				
-3				4	SS	26	-		0	LPID: 0 FID: 0				
				5	SS	33	189 —		0	_PID: 0 FID: 0				
- 4					I		188 –							
- 5				6	SS	25	-		0	_PID: 0 FID: 0				
,							187							
-6	186.1 6.1	SANDY SILT to SAND AND SILT, trace to some clay, trace to some gravel, very dense, reddish brown, moist (GLACIAL TILL)		7	SS	84 / 275mm	186 —		Φ	PID: 0				
-7			· · · · · · · · · · · · · · · · · · ·		1		- 185 -							
- 8		wet below	· · · · ·	8	SS	50 / 150mm	-		0	PID: 0 · · · · · · · · · · · · · · · · · ·				
					1		184							
-9	<u>183.0</u> 9.2	L		<u>e</u>	SS	50 / 100mm			0	PID: 0 FID: 0 FID: 0				
		END OF BOREHOLE Borehole was dry and open upon completion of drilling						WATER LE <sup>1</sup> Date Water Jul 11, 2022 Jul 14, 2022	VEL READINGS Depth (m) Elevation (m 7.1 185.1 7.3 184.9	ŋ				

Borehole was dry and completion of drilling.

50 mm dia. monitoring well installed.

file: 1-22-0209-01 bh logs.gpj














- /				_	Falling	Head Test	Analysis Report	E
	22	<b>T</b> 0	rrad	robe	Project:	Hydrogeolo	ogical assessment	
		Consulting Co	otophning & En	ironmontal Engineer	Number	: 1-22-0209-	46	
		Construction M	aterials Engineeri	ng, Inspection & Te	<sup>sting</sup> Client:	Creswell R	esidences Inc	
Locat	ion: 63	360 Regional F	Road 25, Milton	Slug Test: BH1			Test Well: BH1	
Test (	Condu	cted by:					Test Date: 2022-07-15	
Analy	sis Pe	erformed by: M	M	Falling Head Te	st		Analysis Date: 2022-08-03	
Aquife	er Thio	ckness:						
	(	0	700	1400	Time [s]	2100	2800	3500
	1E1-							
	-							
	-							
	-							
	-							
	-							
)ч/ч	1E0-							
	-							
	-							
	-							
	-							
	1E-1-							
Calcul	ation u	sing Hvorslev						
Observ	vation V	Vell	Hydraulic Condu	uctivity				
			[m/s]					
			3.10 × 10 <sup>-8</sup>					
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								
BH1								

<b>—</b>		_	Slug Te	st Analysi	s Report			
AND TO	rrad	robe	Project: Hydrogeological assessment					
Consulting Co	atachnical & Envir	conmental Engineering	Number: 1-22-0209-46					
Construction Mc	aterials Engineering	g, Inspection & Testing	Client:	Creswell Re	esidences Inc			
Location: 6360 Regional R	Road 25, Milton	Slug Test: BH4			Test Well: BH	4		
Test Conducted by:					Test Date: 20	22-07-15		
Analysis Performed by:	Analysis Performed by: New analysis 1				Analysis Date: 2022-08-03			
Aquifer Thickness:								
		т	ime [s]					
0	320	640		960	12	80	1600	
1E0								
୍ୟୁ 1E-1								
15.2								
					- •			
1E-3								
Calculation using Hvorslev								
Observation Well	Hydraulic Conduc	ctivity						
	[m/s]							
BH4	1.84 × 10 <sup>-6</sup>							

<b>—</b>		_	Slug Te	st Analysi	s Report	E
1222 TO	rrad	robe	Project:	Hydrogeolo	gical assessment	
Consulting Co	stachnical & Envi		Number: 1-22-0209-46			
Construction Mc	iterials Engineerir	ng, Inspection & Testing	Client:	Creswell Re	esidences Inc	
Location: 6360 Regional R	load 25, Milton	Slug Test: BH7			Test Well: BH7	
Test Conducted by:					Test Date: 2022-07-15	1
Analysis Performed by: M	N	Falling Head Test			Analysis Date: 2022-08	3-03
Aquifer Thickness:						
0	800	<b>T</b> 1600	ïme [s]	2400	3200	4000
1E1						
1E0						
아 						
2 						
1E-1-						
1E-2						
Calculation using Hvorslev						
Observation Well	Hydraulic Condu	ctivity				
	[m/s]					
BH7	3.78 × 10 <sup>-7</sup>					

		_	Falling	Head Test	Analysis Report	E
A Torraprobo			Project: Hydrogeological assessment			
			Number: 1-22-0209-46			
Construction Ma	terials Engineering	g, Inspection & Testing	Client:	Creswell R	esidences Inc	
Location: 6360 Regional R	oad 25, Milton	Slug Test: BH9			Test Well: BH9	
Test Conducted by:					Test Date: 2022-07-15	
Analysis Performed by: M	Л	Falling Head Test			Analysis Date: 2022-08-0	)3
Aquifer Thickness:						
		-	lime [s]			
0	700	1400		2100	2800	3500
04/						
2						
1E-1						
Calculation using Hvorslev	1					
Observation Well	Hydraulic Conduc	tivity				
	[m/s]					
BH9	3.51 × 10 <sup>-8</sup>					

			_	Slug T	est Analys	is Report		
Terraprobe			Project	Project: Hydrogeological assessment				
	Consulting Gentechnical & Environmental Engineering			Numbe	Number: 1-22-0209-46			
	Construction Mo	aterials Engineerin	g, Inspection & T	esting Client:	Creswell F	Residences Inc		
Location: 6	360 Regional R	Road 25, Milton	Slug Test: BH1	0		Test Well: BH	110	
Test Condu	icted by:		<b>.</b>			Test Date: 20	22-07-15	
Analysis Pe	erformed by:		New analysis 1			Analysis Date	e: 2022-08-03	
Aquifer Thi	ckness:		•					
				Time [s]				
1E1-	0	800	1600		2400	32	00	4000
-								
<b>) 년</b> 1E0 -	<b>.</b>							
	· · ·							
	*							
-								
-								
1E-1-								
Calculation u	sing Hvorslev							
Observation \	Vell	Hydraulic Condu	ctivity					
		[m/s]						
BH10		2.99 × 10 <sup>-8</sup>						
		1	I					

<b>—</b>		_	Falling H	lead Test	Analysis Repo	ort	E
	rrap	obe	Project: I	Hydrogeolo	gical assessmen	t	
			Number: 7	1-22-0209-4	46		
Consulting Geo Construction Ma	terials Engineering	, Inspection & Testing	Client: (	Creswell Re	esidences Inc		
Location: 6360 Regional R	oad 25. Milton	Slua Test: BH13			Test Well: BH13	3	
Test Conducted by:		5			Test Date: 2022	2-07-15	
Analysis Performed by: MM	Λ	Falling Head Test			Analysis Date: 2	2022-08-03	
Aquifer Thickness:	L. C.						
0	640	<b>T</b> 1280	ime [s]	1920	2560	3	200
1E1							
							_
•							
							_
							_
							_
							_
1E-1							
Calculation using Hvorslev							
Observation Well	Hydraulic Conduct	ivity					
	[m/s]						
BH13	1.18 × 10 <sup>-8</sup>						

		_	Falling I	lead Test	Analysis Report	E
	rrapr	nbo	Project:	Hydrogeolc	gical assessment	
			Number:	1-22-0209-	46	
Consulting Ge Construction M	aterials Engineering,	mental Engineering Inspection & Testing	Client:	Creswell R	esidences Inc	
Location: 6360 Regional F	Road 25 Milton S	lua Test: BH15	-		Test Well: BH15	
Test Conducted by:					Test Date: 2022-07-15	
Analysis Performed by: M	M F	alling Head Test			Analysis Date: 2022-08-0	03
Aquifer Thickness:		5			,	
0	640	<b>T</b> 1280	ime [s]	1920	2560	3200
1E1						
9						
1E-1						
Calculation using Hvorslev						
Observation Well	Hydraulic Conductiv	ity				
	[m/s]					
BH15	1.67 × 10 <sup>-7</sup>					

		_	Slug Te	est Analys	is Report		E
A Terraprobe			Project:	Project: Hydrogeological assessment			
Consulting Co	atachnical & Envi		Number	Number: 1-22-0209-46			
Construction Mc	iterials Engineerin	g, Inspection & Testi	<sup>ng</sup> Client:	Creswell F	Residences Inc		
Location: 6360 Regional R	load 25, Milton	Slug Test: BH16			Test Well: BH	16	
Test Conducted by:					Test Date: 202	22-07-15	
Analysis Performed by: M	N	Falling Head Test			Analysis Date:	2022-08-03	
Aquifer Thickness:							
			Time [s]				
0	500	1000		1500	200	0	2500
1E0							
아 ~							
1E-1							
1E-2							]
Calculation using Hvorslev							
Observation Well	Hydraulic Condu	ctivity					
	[m/s]						
BH16	3.49 × 10 <sup>-7</sup>						



### Poorly sorted sandy gravelly silt with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	7.9E-07	7.9E-09	0.00	
Hazen K (cm/s) = $d_{10}$ (mm)	1.4E-06	1.4E-08	0.00	
Slichter	1.6E-07	1.6E-09	0.00	
Terzaghi	2.2E-07	2.2E-09	0.00	
Beyer	6.2E-07	6.2E-09	0.00	
Sauerbrei	1.7E-06	1.7E-08	0.00	
Kruger	6.4E-05	6.4E-07	0.06	
Kozeny-Carmen	1.5E-05	1.5E-07	0.01	
Zunker	1.1E-05	1.1E-07	0.01	
Zamarin	1.4E-05	1.4E-07	0.01	
USBR	3.1E-06	3.1E-08	0.00	
Barr	1.7E-07	1.7E-09	0.00	
Alyamani and Sen	3.6E-06	3.6E-08	0.00	
Chapuis	3.5E-09	3.5E-11	0.00	
Krumbein and Monk	3.6E-05	3.6E-07	0.03	
geometric mean	9.9E-07	9.9E-09	0.00	
arithmetic mean	1.8E-06	1.8E-08	0.00	

- monland	K from Grain Size Analysis Rep	port	Date:	11-Aug-22
XL	Sample Name:	BH4 SS6		
Sieve		100	T (aC)	20
	Mass Sample (g):	100	I (OC)	20

### Poorly sorted clay with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	2.2E-07	2.2E-09	0.00	
Hazen K (cm/s) = $d_{10}$ (mm)	3.8E-07	3.8E-09	0.00	
Slichter	4.3E-08	4.3E-10	0.00	
Terzaghi	6.1E-08	6.1E-10	0.00	
Beyer	2.4E-07	2.4E-09	0.00	
Sauerbrei	1.3E-07	1.3E-09	0.00	
Kruger	2.1E-05	2.1E-07	0.02	
Kozeny-Carmen	4.6E-06	4.6E-08	0.00	
Zunker	3.5E-06	3.5E-08	0.00	
Zamarin	4.1E-06	4.1E-08	0.00	
USBR	9.7E-08	9.7E-10	0.00	
Barr	4.6E-08	4.6E-10	0.00	
Alyamani and Sen	6.2E-07	6.2E-09	0.00	
Chapuis	5.8E-10	5.8E-12	0.00	
Krumbein and Monk	2.0E-05	2.0E-07	0.02	
geometric mean	1.5E-07	1.5E-09	0.00	
arithmetic mean	2.7E-07	2.7E-09	0.00	

androge	K from Grain Size Analysis Re	port	Date: 11-Aug-22		
XL	Sample Name:	BH10 SS5			
Sieve					
	Mass Sample (g):	100	Т (оС)	20	

## Poorly sorted sandy silt with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	2.7E-07	2.7E-09	0.00	
Hazen K (cm/s) = $d_{10}$ (mm)	4.8E-07	4.8E-09	0.00	
Slichter	5.3E-08	5.3E-10	0.00	
Terzaghi	7.6E-08	7.6E-10	0.00	
Beyer	2.4E-07	2.4E-09	0.00	
Sauerbrei	1.6E-07	1.6E-09	0.00	
Kruger	3.3E-05	3.3E-07	0.03	
Kozeny-Carmen	6.2E-06	6.2E-08	0.01	
Zunker	4.7E-06	4.7E-08	0.00	
Zamarin	5.5E-06	5.5E-08	0.00	
USBR	1.8E-07	1.8E-09	0.00	
Barr	5.7E-08	5.7E-10	0.00	
Alyamani and Sen	1.2E-06	1.2E-08	0.00	
Chapuis	7.8E-10	7.8E-12	0.00	
Krumbein and Monk	2.3E-05	2.3E-07	0.02	
geometric mean	2.2E-07	2.2E-09	0.00	
arithmetic mean	4.6E-07	4.6E-09	0.00	



### Poorly sorted sandy silt with fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	9.0E-07	9.0E-09	0.00	
Hazen K (cm/s) = $d_{10}$ (mm)	1.6E-06	1.6E-08	0.00	
Slichter	1.8E-07	1.8E-09	0.00	
Terzaghi	2.5E-07	2.5E-09	0.00	
Beyer	7.5E-07	7.5E-09	0.00	
Sauerbrei	1.8E-06	1.8E-08	0.00	
Kruger	5.9E-05	5.9E-07	0.05	
Kozeny-Carmen	1.5E-05	1.5E-07	0.01	
Zunker	1.2E-05	1.2E-07	0.01	
Zamarin	1.4E-05	1.4E-07	0.01	
USBR	3.2E-06	3.2E-08	0.00	
Barr	1.9E-07	1.9E-09	0.00	
Alyamani and Sen	3.1E-06	3.1E-08	0.00	
Chapuis	4.3E-09	4.3E-11	0.00	
Krumbein and Monk	4.5E-05	4.5E-07	0.04	
geometric mean	1.0E-06	1.0E-08	0.00	
arithmetic mean	1.7E-06	1.7E-08	0.00	



### Poorly sorted gravelly sand low in fines



Estimation of Hydraulic Conductivity	cm/s	m/s	m/d	de
Hazen	2.4E-05	2.4E-07	0.02	
Hazen K (cm/s) = $d_{10}$ (mm)	4.3E-05	4.3E-07	0.04	
Slichter	4.7E-06	4.7E-08	0.00	
Terzaghi	6.8E-06	6.8E-08	0.01	
Beyer	2.1E-05	2.1E-07	0.02	
Sauerbrei	2.3E-05	2.3E-07	0.02	
Kruger	2.3E-04	2.3E-06	0.20	
Kozeny-Carmen	6.4E-05	6.4E-07	0.06	
Zunker	4.9E-05	4.9E-07	0.04	
Zamarin	5.7E-05	5.7E-07	0.05	
USBR	5.3E-05	5.3E-07	0.05	
Barr	5.1E-06	5.1E-08	0.00	
Alyamani and Sen	3.0E-05	3.0E-07	0.03	
Chapuis	4.4E-07	4.4E-09	0.00	
Krumbein and Monk	2.8E-04	2.8E-06	0.24	
geometric mean	3.1E-05	3.1E-07	0.03	
arithmetic mean	8.4E-05	8.4E-07	0.07	





### CLIENT NAME: TERRAPROBE INC. 11 INDELL LANE BRAMPTON, ON L6T3Y3 (905) 796-2650 ATTENTION TO: Justin Rumney PROJECT: 1-22-0209-46 AGAT WORK ORDER: 22T921250 MICROBIOLOGY ANALYSIS REVIEWED BY: Nivine Basily, Inorganics Report Writer TRACE ORGANICS REVIEWED BY: Nivine Basily, Inorganics Report Writer WATER ANALYSIS REVIEWED BY: Pinkal Patel, Report Reviewer WATER ANALYSIS REVIEWED BY: Yris Verastegui, Report Reviewer DATE REPORTED: Jul 25, 2022 PAGES (INCLUDING COVER): 11 VERSION\*: 1

Should you require any information regarding this analysis please contact your client services representative at (905) 712-5100

\*Notes

Disclaimer:

- All work conducted herein has been done using accepted standard protocols, and generally accepted practices and methods. AGAT test methods may
  incorporate modifications from the specified reference methods to improve performance.
- All samples will be disposed of within 30 days after receipt unless a Long Term Storage Agreement is signed and returned. Some specialty analysis may be exempt, please contact your Client Project Manager for details.
- AGAT's liability in connection with any delay, performance or non-performance of these services is only to the Client and does not extend to any other third party. Unless expressly agreed otherwise in writing, AGAT's liability is limited to the actual cost of the specific analysis or analyses included in the services.
- This Certificate shall not be reproduced except in full, without the written approval of the laboratory.
- The test results reported herewith relate only to the samples as received by the laboratory.
- Application of guidelines is provided "as is" without warranty of any kind, either expressed or implied, including, but not limited to, warranties of
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  contained in this document.
- All reportable information as specified by ISO/IEC 17025:2017 is available from AGAT Laboratories upon request.

**AGAT** Laboratories (V1)

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AGAT WORK ORDER: 22T921250 PROJECT: 1-22-0209-46 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

### CLIENT NAME: TERRAPROBE INC.

SAMPLING SITE:6360 Regional Rd, Milton

ATTENTION TO: Justin Rumney

SAMPLED BY:DM

				E. C	Coli (Using MI Agar)
DATE RECEIVED: 2022-07-7	15				DATE REPORTED: 2022-07-25
	SA	MPLE DES	CRIPTION:	BH16	
		SAM	PLE TYPE:	Water	
		DATE	SAMPLED:	2022-07-15 15:00	
Parameter	Unit	G/S	RDL	4100481	
Escherichia coli	CFU/100mL	200		0	

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Halton Storm Sewer

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

4100481 Escherichia coli RDL = 1 CFU/100mL.

Analysis performed at AGAT Toronto (unless marked by \*)



Certified By:



AGAT WORK ORDER: 22T921250 PROJECT: 1-22-0209-46 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

#### CLIENT NAME: TERRAPROBE INC.

#### SAMPLING SITE:6360 Regional Rd, Milton

ATTENTION TO: Justin Rumney

SAMPLED BY:DM

#### Halton Sanitary and Combined Sewer Use By-law - Organics DATE RECEIVED: 2022-07-15 DATE REPORTED: 2022-07-25 SAMPLE DESCRIPTION: BH16 SAMPLE TYPE: Water DATE SAMPLED: 2022-07-15 15:00 Parameter Unit G/S RDL 4100481 Oil and Grease (animal/vegetable) mg/L 150 0.5 <0.5 in water Oil and Grease (mineral) in water mg/L 15 0.5 <0.5 2 Methylene Chloride mg/L 0.0003 < 0.0003 Chloroform 0.0002 < 0.0002 mg/L 0.04 Benzene mg/L 0.0002 < 0.0002 Trichloroethene mg/L 0.4 0.0002 < 0.0002 Toluene 0.0002 < 0.0002 mg/L 0.016 Tetrachloroethene < 0.010 mg/L 1 0.010 Ethylbenzene mg/L 0.16 0.0001 < 0.0001 1,4-Dichlorobenzene mg/L 0.08 0.0002 < 0.0002 Naphthalene mg/L 0.14 0.0003 < 0.0003 Surrogate Unit Acceptable Limits Toluene-d8 % Recovery 50-140 100 103 4-Bromofluorobenzene % Recovery 50-140 Naphthalene-d8 % 50-140 92

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Halton Sanitary & Combined Sewer

Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation. Oil and Grease animal/vegetable is a calculated parameter. The calculated value is the difference between Total O&G and Mineral O&G.

Analysis performed at AGAT Toronto (unless marked by \*)

I'mkal Jata

Certified By:

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AGAT WORK ORDER: 22T921250 PROJECT: 1-22-0209-46 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

#### CLIENT NAME: TERRAPROBE INC.

#### SAMPLING SITE:6360 Regional Rd, Milton

ATTENTION TO: Justin Rumney

SAMPLED BY:DM

#### Halton Sanitary and Combined Sewer Use By-law - Inorganics DATE RECEIVED: 2022-07-15 DATE REPORTED: 2022-07-25 SAMPLE DESCRIPTION: BH16 SAMPLE TYPE: Water DATE SAMPLED: 2022-07-15 15:00 Parameter Unit G/S RDL 4100481 pH Units NA 7.78 bН 6.0-10.0 CBOD (5) mg/L 300 2 <2 Total Suspended Solids 350 10 33 mg/L Fluoride mg/L 10 0.05 < 0.05 Sulphate mg/L 1500 0.10 68.0 Cyanide, SAD mg/L 2 0.002 < 0.002 1 0.005 Phenols mg/L 0.004 Total Kjeldahl Nitrogen mg/L 100 0.10 0.12 Total Phosphorus mg/L 10 0.02 0.05 Total Aluminum 50 mg/L 0.010 1.15 5 Total Antimony mg/L 0.020 <0.020 0.015 <0.015 Total Arsenic mg/L 1 5 0.001 < 0.001 Total Beryllium mg/L Total Cadmium mg/L 1 <0.010 0.010 3 Total Chromium mg/L 0.020 < 0.020 Total Cobalt mg/L 5 0.020 < 0.020 3 Total Copper mg/L 0.015 < 0.015 Total Iron 50 mg/L 0.050 1.54 Total Lead mg/L 3 0.020 < 0.020 5 Total Manganese mg/L 0.020 0.240 0.05 0.0002 < 0.0002 Total Mercury mg/L 5 0.020 < 0.020 Total Molybdenum mg/L 3 Total Nickel mg/L 0.015 < 0.015 Total Selenium mg/L 5 0.002 0.003 Total Silver mg/L 5 0.020 < 0.020 Total Tin mg/L 5 0.025 <0.025 Total Titanium 5 0.010 0.020 mg/L 3 Total Zinc mg/L 0.020 < 0.020

Certified By:

Irús Verastegui



AGAT WORK ORDER: 22T921250 PROJECT: 1-22-0209-46 5835 COOPERS AVENUE MISSISSAUGA, ONTARIO CANADA L4Z 1Y2 TEL (905)712-5100 FAX (905)712-5122 http://www.aqatlabs.com

#### CLIENT NAME: TERRAPROBE INC.

SAMPLING SITE:6360 Regional Rd, Milton

ATTENTION TO: Justin Rumney

SAMPLED BY:DM

### Halton Sanitary and Combined Sewer Use By-law - Inorganics

DATE REPORTED: 2022-07-25

Comments: RDL - Reported Detection Limit; G / S - Guideline / Standard: Refers to Halton Sanitary & Combined Sewer Guideline values are for general reference only. The guidelines provided may or may not be relevant for the intended use. Refer directly to the applicable standard for regulatory interpretation.

4100481 Dilution required, RDL has been increased accordingly.

Analysis performed at AGAT Toronto (unless marked by \*)

Certified By:

Inis Verastegui



### **Quality Assurance**

#### CLIENT NAME: TERRAPROBE INC.

#### PROJECT: 1-22-0209-46

SAMPLING SITE:6360 Regional Rd, Milton

AGAT WORK ORDER: 22T921250

**ATTENTION TO: Justin Rumney** 

SAMPLED BY:DM

			Mic	crobi	ology	y Ana	alysis	5							
RPT Date: Jul 25, 2022			0	UPLICAT	E		REFEREN	NCE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	KE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lin	ptable nits	Recovery	Acce Lir	ptable nits	Recovery	Acce Lin	ptable nits
		Ia					value	Lower	Upper		Lower	Upper		Lower	Upper
E. Coli (Using MI Agar)															

Escherichia coli 4100481 4100481 0 0 NA

Comments: NA - % RPD Not Applicable.





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AGAT QUALITY ASSURANCE REPORT (V1)

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### **Quality Assurance**

#### CLIENT NAME: TERRAPROBE INC.

#### PROJECT: 1-22-0209-46

#### SAMPLING SITE:6360 Regional Rd, Milton

#### AGAT WORK ORDER: 22T921250

#### **ATTENTION TO: Justin Rumney**

#### SAMPLED BY:DM

			Trac	e Org	ganio	cs An	alysi	is							
RPT Date: Jul 25, 2022			۵	UPLICATE			REFEREN	ICE MA	TERIAL	METHOD	BLANK	SPIKE	MAT	RIX SPI	КЕ
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lir	ptable nits	Recoverv	Acce Lin	ptable nits	Recoverv	Acce Lin	ptable nits
		Id					value	Lower	Upper		Lower	Upper		Lower	Upper
Halton Sanitary and Combined Se	ewer Use B	y-law - Or	ganics												
Oil and Grease (animal/vegetable) in water	4087752		0.71	0.69	NA	< 0.5	87%	60%	130%	101%	60%	130%	98%	60%	130%
Oil and Grease (mineral) in water	4087752		< 0.5	< 0.5	0.0%	< 0.5	81%	60%	130%	90%	60%	130%	84%	60%	130%
Methylene Chloride	4101149		< 0.0003	< 0.0003	0.0%	< 0.0003	76%	50%	140%	71%	60%	130%	70%	50%	140%
Chloroform	4101149		< 0.0002	< 0.0002	0.0%	< 0.0002	87%	50%	140%	87%	60%	130%	86%	50%	140%
Benzene	4101149		< 0.0002	< 0.0002	0.0%	< 0.0002	114%	50%	140%	76%	60%	130%	78%	50%	140%
Trichloroethene	4101149		< 0.0002	< 0.0002	0.0%	< 0.0002	97%	50%	140%	95%	60%	130%	101%	50%	140%
Toluene	4101149		< 0.0002	< 0.0002	0.0%	< 0.0002	100%	50%	140%	93%	60%	130%	93%	50%	140%
Tetrachloroethene	4101149		< 0.010	< 0.010	0.0%	< 0.010	98%	50%	140%	91%	60%	130%	93%	50%	140%
Ethylbenzene	4101149		< 0.0001	< 0.0001	0.0%	< 0.0001	104%	50%	140%	97%	60%	130%	96%	50%	140%
1,4-Dichlorobenzene	4101149		< 0.0002	< 0.0002	0.0%	< 0.0002	112%	50%	140%	103%	60%	130%	94%	50%	140%
Naphthalene	4092744		< 0.0003	< 0.0003	0.0%	< 0.0003	104%	50%	140%	81%	50%	140%	87%	50%	140%

Comments: When the average of the sample and duplicate results is less than 5x the RDL, the Relative Percent Difference (RPD) will be indicated as Not Applicable (NA).

Certified By:

Imkal Jata

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**AGAT** QUALITY ASSURANCE REPORT (V1)

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## **Quality Assurance**

#### CLIENT NAME: TERRAPROBE INC.

#### PROJECT: 1-22-0209-46

#### SAMPLING SITE:6360 Regional Rd, Milton

#### AGAT WORK ORDER: 22T921250

#### **ATTENTION TO: Justin Rumney**

#### SAMPLED BY:DM

				Wate	er Ar	nalysi	is								
RPT Date: Jul 25, 2022			C	UPLICATE	=		REFERE	NCE MA	TERIAL	METHOD	BLAN	K SPIKE	MAT	RIX SPI	IKE
PARAMETER	Batch	Sample	Dup #1	Dup #2	RPD	Method Blank	Measured	Acce Lir	ptable nits	Recovery	Acce Lir	eptable mits	Recovery	Acce	ptable nits
		iu iu					Value	Lower	Upper		Lower	Upper		Lower	Upper
Halton Sanitary and Combined Se	ewer Use E	By-law - In	organics												
рН	4099156		7.50	7.65	2.0%	NA	101%	90%	110%						
CBOD (5)	4101058		5	5	NA	< 2	101%	75%	125%						
Total Suspended Solids	4100481	4100481	33	33	NA	< 10	96%	80%	120%						
Fluoride	4099255		<0.05	<0.05	NA	< 0.05	103%	70%	130%	104%	80%	120%	101%	70%	130%
Sulphate	4099255		1.72	1.64	4.8%	< 0.10	101%	70%	130%	102%	80%	120%	101%	70%	130%
Cyanide, SAD	4082679		<0.002	<0.002	NA	< 0.002	96%	70%	130%	86%	80%	120%	101%	70%	130%
Phenols	4099143		0.009	0.008	NA	< 0.002	100%	90%	110%	101%	90%	110%	107%	80%	120%
Total Kjeldahl Nitrogen	4099706		<0.10	<0.10	NA	< 0.10	103%	70%	130%	95%	80%	120%	102%	70%	130%
Total Phosphorus	4108521		0.13	0.14	7.4%	< 0.02	99%	70%	130%	98%	80%	120%	NA	70%	130%
Total Aluminum	4101058		0.144	0.160	10.5%	< 0.010	98%	70%	130%	103%	80%	120%	110%	70%	130%
Total Antimony	4101058		<0.020	<0.020	NA	< 0.020	100%	70%	130%	100%	80%	120%	106%	70%	130%
Total Arsenic	4101058		<0.015	<0.015	NA	< 0.015	94%	70%	130%	97%	80%	120%	105%	70%	130%
Total Beryllium	4101058		<0.001	<0.001	NA	< 0.001	99%	70%	130%	106%	80%	120%	107%	70%	130%
Total Cadmium	4101058		<0.010	<0.010	NA	< 0.010	98%	70%	130%	100%	80%	120%	103%	70%	130%
Total Chromium	4101058		<0.020	<0.020	NA	< 0.020	100%	70%	130%	105%	80%	120%	106%	70%	130%
Total Cobalt	4101058		<0.020	<0.020	NA	< 0.020	99%	70%	130%	105%	80%	120%	104%	70%	130%
Total Copper	4101058		<0.015	<0.015	NA	< 0.015	100%	70%	130%	102%	80%	120%	99%	70%	130%
Total Iron	4101058		1.37	1.49	8.4%	< 0.050	97%	70%	130%	103%	80%	120%	112%	70%	130%
Total Lead	4101058		<0.020	<0.020	NA	< 0.020	96%	70%	130%	97%	80%	120%	100%	70%	130%
Total Manganese	4101058		0.522	0.559	6.8%	< 0.020	101%	70%	130%	105%	80%	120%	101%	70%	130%
Total Mercury	4101058		<0.0002	<0.0002	NA	< 0.0002	101%	70%	130%	101%	80%	120%	90%	70%	130%
Total Molybdenum	4101058		0.020	0.022	NA	< 0.020	100%	70%	130%	108%	80%	120%	111%	70%	130%
Total Nickel	4101058		0.016	<0.015	NA	< 0.015	100%	70%	130%	105%	80%	120%	103%	70%	130%
Total Selenium	4101058		0.007	0.008	NA	< 0.002	103%	70%	130%	103%	80%	120%	111%	70%	130%
Total Silver	4101058		<0.020	<0.020	NA	< 0.020	99%	70%	130%	102%	80%	120%	98%	70%	130%
Total Tin	4101058		<0.025	<0.025	NA	< 0.025	102%	70%	130%	101%	80%	120%	103%	70%	130%
Total Titanium	4101058		<0.010	<0.010	NA	< 0.010	98%	70%	130%	106%	80%	120%	114%	70%	130%
Total Zinc	4101058		0.028	0.028	NA	< 0.020	100%	70%	130%	101%	80%	120%	109%	70%	130%

Comments: NA signifies Not Applicable.

If the RPD value is NA, the results of the duplicates are under 5X the RDL and will not be calculated.

Matrix spike NA: Spike level < native concentration. Matrix spike acceptance limits do not apply and are not calculated.

Certified By:

Inis Verastegui

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# Method Summary

CLIENT NAME: TERRAPROBE INC.

#### PROJECT: 1-22-0209-46

SAMPLING SITE:6360 Regional Rd, Milton

AGAT WORK ORDER: 22T921250

**ATTENTION TO: Justin Rumney** 

SAMPLING SITE:6360 Regional Rd, Milto	n	SAMPLED BY:DN	1
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Microbiology Analysis		•	
Escherichia coli	MIC-93-7010	EPA 1604	Membrane Filtration
Trace Organics Analysis			
Oil and Grease (animal/vegetable) in water	VOL-91-5011	EPA SW-846 3510C & SM5520	BALANCE
Oil and Grease (mineral) in water	VOL-91-5011	EPA SW-846 3510C & SM5520	BALANCE
Methylene Chloride	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Chloroform	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Benzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Trichloroethene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Toluene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Tetrachloroethene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Ethylbenzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
1,4-Dichlorobenzene	VOL-91-5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Naphthalene	ORG-91-5105	modified from EPA 3510C and EPA 8270E	GC/MS
Toluene-d8	VOL-91- 5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
4-Bromofluorobenzene	VOL-91- 5001	modified from EPA 5030B & EPA 8260D	(P&T)GC/MS
Naphthalene-d8	ORG-91-5105	modified from EPA 3510C and EPA 8270E	GC/MS



# Method Summary

CLIENT NAME: TERRAPROBE INC.

#### PROJECT: 1-22-0209-46

AGAT WORK ORDER: 22T921250

ATTENTION TO: Justin Rumney

SAMPLING SITE:6360 Regional Rd	, Milton	SAMPLED BY:DN	Λ
PARAMETER	AGAT S.O.P	LITERATURE REFERENCE	ANALYTICAL TECHNIQUE
Water Analysis			
pH	INOR-93-6000	modified from SM 4500-H+ B	PC TITRATE
CBOD (5)	INOR-93-6006	Modified from SM 5210 B	DO METER
Total Suspended Solids	INOR-93-6028	modified from EPA 1684,ON MOECC E3139,SM 2540C,D	BALANCE
Fluoride	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Sulphate	INOR-93-6004	modified from SM 4110 B	ION CHROMATOGRAPH
Cyanide, SAD	INOR-93-6051	modified from MOECC E3015; SM 4500-CN- A, B, & C	TECHNICON AUTO ANALYZER
Phenols	INOR-93-6072	modified from SM 5530 D	LACHAT FIA
Total Kjeldahl Nitrogen	INOR-93-6048	modified from EPA 351.2 and SM 4500-NORG D	LACHAT FIA
Total Phosphorus	INOR-93-6022	modified from SM 4500-P B and SM 4500-P E	SPECTROPHOTOMETER
Total Aluminum	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Antimony	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Arsenic	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Beryllium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Cadmium	MET -93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Chromium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Cobalt	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Copper	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Iron	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Lead	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Manganese	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Mercury	MET-93-6100	modified from EPA 245.2 and SM 311 B	<sup>2</sup> CVAAS
Total Molybdenum	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Nickel	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Selenium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Silver	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Tin	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Titanium	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS
Total Zinc	MET-93-6103	modified from EPA 200.8, 3005A, 3010A & 6020B	ICP-MS

Chain of Custody Reco	Cal.	Lal	DOra	torie	Ph: <b>In this Water Chain of Custody Form</b> (poted)	M : 905.71	ississa L2.510 v	5835 auga, ( )0 Fax vebear	Coope Ontaric :: 905. th.aga	L4Z 1 L4Z 1 712 51 Labs co	uc Y2 22 om		<b>_aborato</b> Vork Order # Cooler Quant	ry Use : ity: iratures:	• On 2 T 1 C	ly 92 -9 20	12	50	16	5
Report Information:         Company:       Terraprope Integration         Contact:       Testum Rumme         Address:       II Indell Integration         Address:       II Indell Integration         Phone:       905-796-200         Reports to be sent to:       2rummey @ lest         1. Email:       2rummey @ lest         2. Email:       905-796-200         Project Information:       900 - 100         Project:       1-22-0209-         Site Location:       6360         Region:       100	y   Almae , Bramfr 50 Fax: rafrobe y 6 y 6	l Aswar on , onta ca	Pri 0	Reg (Please Ta Ta Soil T Soil T Soil T Soil T	gulatory Requirements:         e check all applicable boxes)         tegulation 153/04         able	Re Cer	Prc Ob	wer Us Sanitar Rapi Rapi Rapi Rapi Rapi Rapi Rapi Rapi	eer Qua so (PWC) e One deline f Ana	ity 20) e on <i>lysis</i> No		Ti Ri Ri	Custody Sea Notes: Urnaroum egular TA' ush TAT (Ru Days OR Da Ple *TAT is For 'Same	d Tim d Tim ness ate Requ ease prov exclusive Day' ana	e (T/ ages Appl irred (F vide pr e of w livsis,	Yes 2 5 to 7 E y 2 Busir Days Rush Sur rior notificeekends please of	equire Busines hess charge cation and st contact	ed: ss Days s May for rus tatutor	Next E Day Apply): h TAT y holid	UN/A Business
Sampled By: AGAT Quote #: Please note: If quotation number Invoice Information: Company: Contact: Address: Email: Linewice Lessing Contexts	PO:PO: er is not provided, client with Tint . Rafi Raphope	be billed full price for a ill To Same: Yes m, on L	nalysis. No 🗆	Sam B GW O P S SD SW	nple Matrix Legend Biota Ground Water Oil Paint Soil Sediment Surface Water	Field Filtered - Metals, Hg, CrVI, DOC	s & Inorganics	D. Reg 1 B. CLVI, □ Hg, □ HSWH □ , INTO □ - 2	F1-F4 PHCs			1	II Disposal Characterization TCLP: 99, 20, 20, 20, 20, 20, 20, 20, 20, 20, 20	LI Metals LI VOCS LI SVOCS Ba S Soils Characterization Package 906 PMS Metals BTFX F1-F4	sivity: Include Moisture 🗆 Sulphide	wer let				ally Hazardous or High Concentration (Y/N)
Sample Identification	Date Sampled	Time Sampled 3.'00 AM PM AM PM AM PM AM PM	# of Containers 20	Sample Matrix	Comments/ Special Instructions	Y/N	Metal	Metal	BTEX	PAHS	NOC	Aroclo	Landi TCLP: L Exces	Exces	Corros	×				Potenti
Samples Relinquished By (Print Name and Sign):		AM PM AM PM AM PM AM PM AM AM PM Date	22 Time 22 3:0	00	Samplein Barenved By (Print, Name and Sign):	Riv					ato		Time				22 J	UL 1	5	4=01;
Samples Relinquished By (Print Name and Sign):		Date	Time		Sample's received by (Print Name and Sign):					D	ato ato		Time		Nº:	Page	• 13	of	534	4

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1	Issued for Geotechnical RFQ	2022_02_04
No.	Revision	Date

Orientation



All dimensions to be checked and verified on the job by the Contractor. Any discrepancies are to be reported to the Consultant prior to action. Only the latest approved drawings to be used for construction in conformance with all applicable codes, by-laws and regulations. All drawings remain the property of the Consultant.

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salterpilon

151 Ferris Lane, Suite 400 Barrie, Ontario L4M 6C1 salterpilon.com t: 705.737.3530

Project Information Careswell Milton

6360 Regional Rd 25

Excelligent Care

Drawing Title

Scale 1 : 200

For

# Site Plan - Proposed Borehole Locations

Date 2022\_02\_04 Project No Drawing No

21040 RFQ